



Exploiting the Resilience of Masonry Arch Bridge Infrastructure
Workshop, London 5-6 Sept. 2023

Multi-level assessment of masonry arch bridges

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Department of Civil and Environmental Engineering

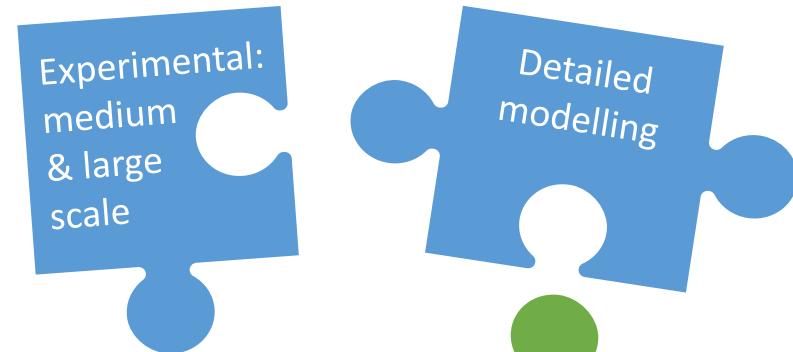
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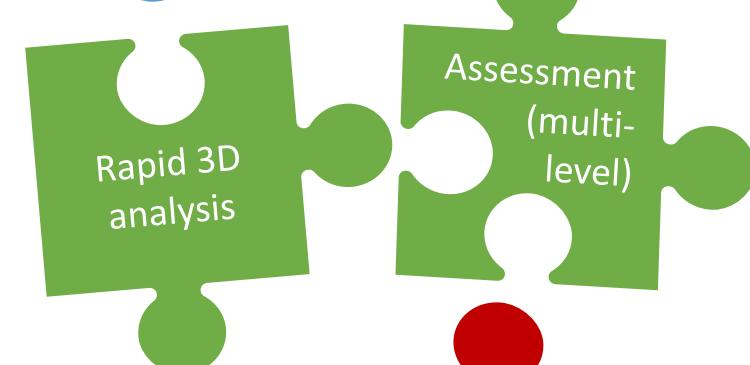


ERMABI: project context

Understanding:



Practical tools:



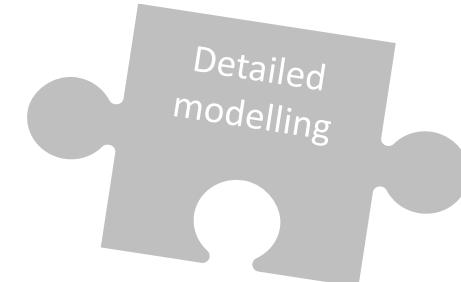
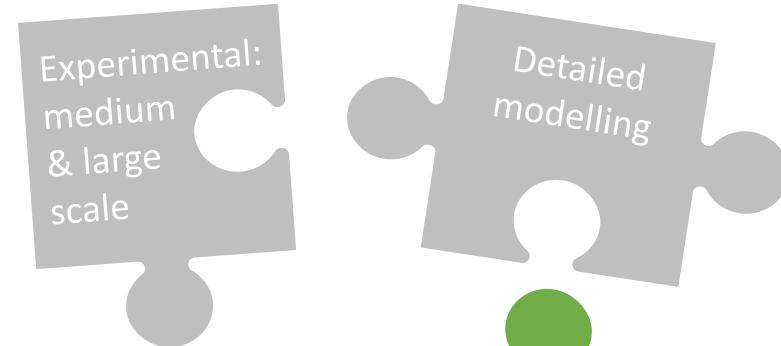
Impact:





ERMABI: project context

Understanding:



Practical tools:



Impact:





Outline

- Background
- Computational strategies for masonry arch bridges
- Parametric study details
- Numerical results
- Conclusions

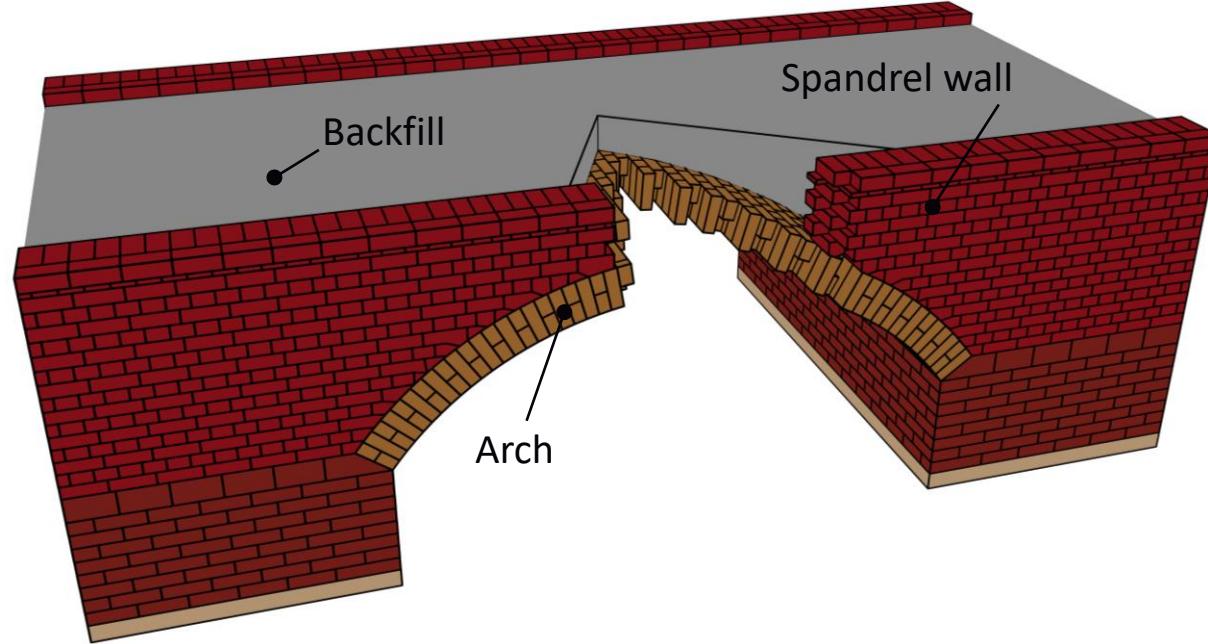
Background



Real bridge behaviour

Masonry arch bridges are inherently three-dimensional, and their response depends upon the interaction between different components, e.g:

- Masonry arch(es)
- Masonry spandrel walls
- Backfill



Accurate 3D models can explicitly represent the different structural and non-structural parts, their nonlinear response and mutual interactions



Multi-level assessment

Increasing sophistication

Table 4.2 Multi-level assessment levels and recommended idealisations

Level	Description	Geometry	Materials	Loading	Calculation tools
0 ¹	Highly simplified	Approximated	Assumed representative	Assumed representative	N/A
1	Simple, where possible based on demonstrably conservative assumptions	Measured (longitudinal slice)	Assumed representative	Assumed representative, taking due account of foreseeable loading patterns	Usually 2D analysis, to model likely global response
2	Escalated	Measured (several longitudinal slices)	Assumed representative and/or measured mechanical properties ²	Current and planned actual	Usually 2D or simplified 3D analysis, both capable of taking account of local details
3	Advanced	Measured	Measured mechanical properties ²	Current and planned actual	Full 3D analysis, capable of capturing key aspects of behaviour

Notes

- 1 Used by some organisations as a sift to identify bridges to be prioritised for more in-depth assessment. However, it may not offer much time saving compared with a Level 1 assessment.
- 2 If necessary derived from a population of similar bridges.



Multi-level assessment

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Approach:
start with level 3
& then reduce
complexity
(for levels 2, 1)



Multi-level assessment

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Main focus of
this presentation



Multi-level assessment

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Ultimate aim is
to cover
Levels 1-3

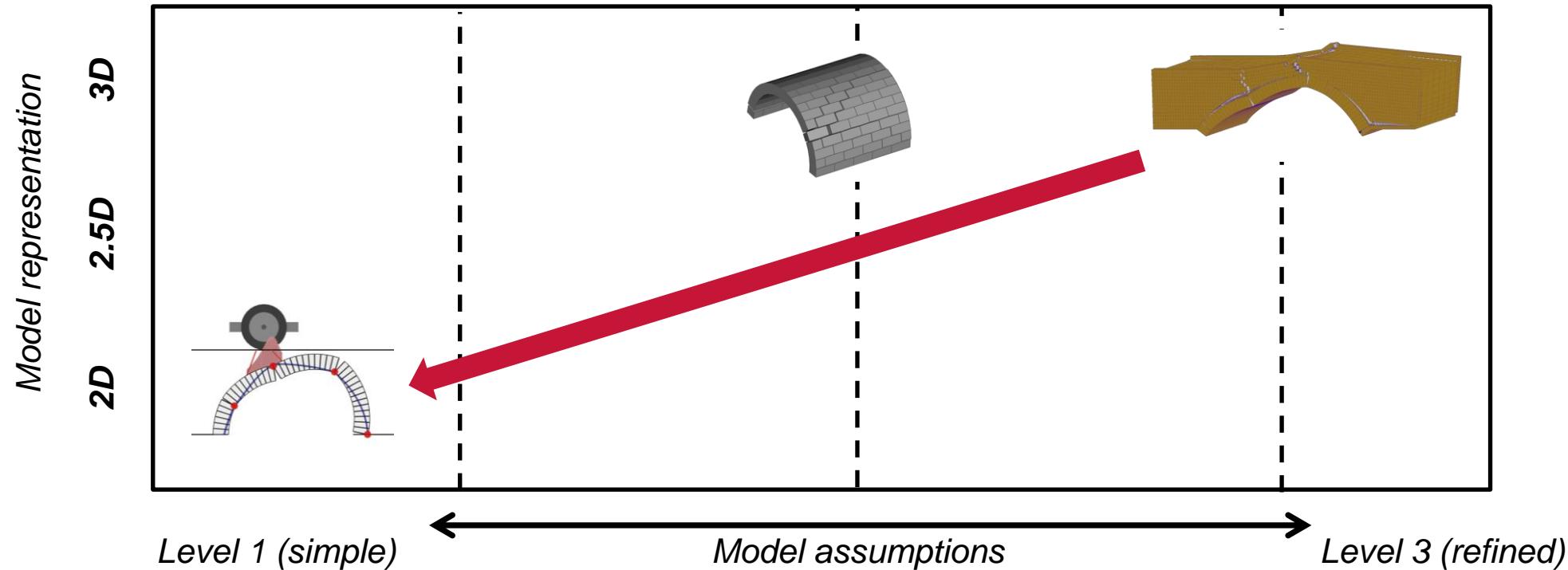


Simplified models

Current Level 1 tools incorporate 2D arch-backfill models, used in conjunction with an assumed effective width.

Aim of Level 1 tools is generally not to model the actual response, but to obtain a reasonable prediction of capacity.

However, currently some assumptions used in Level 1 tools are non-conservative.



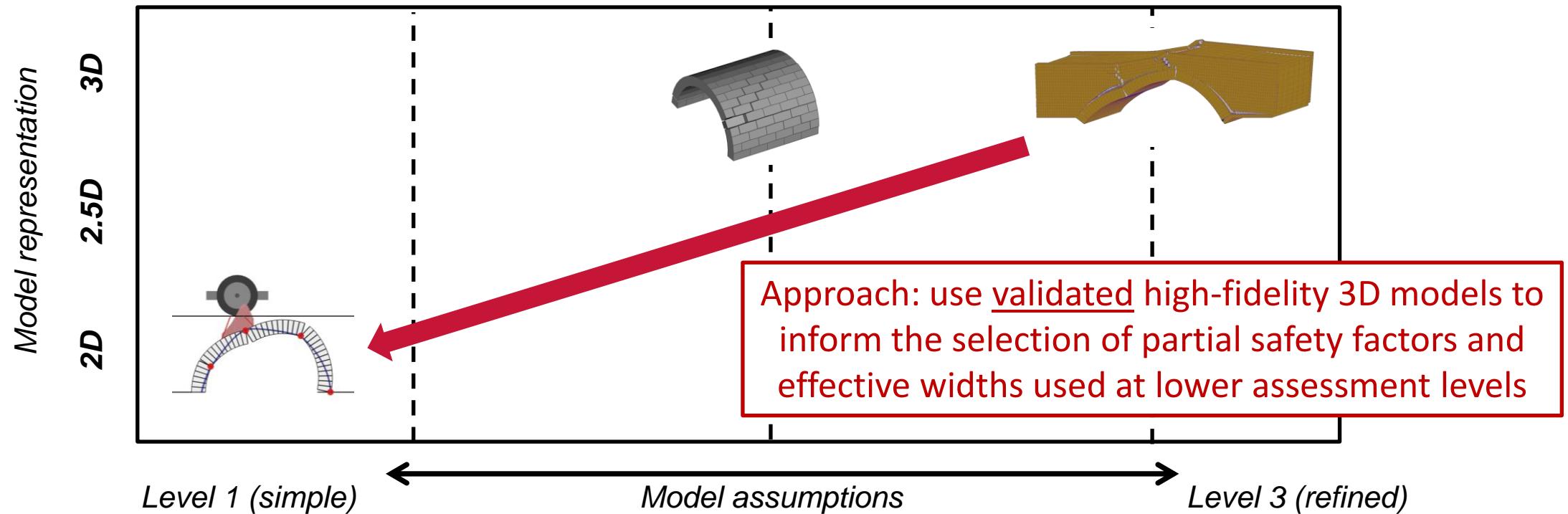


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Computational strategies for masonry arch bridges



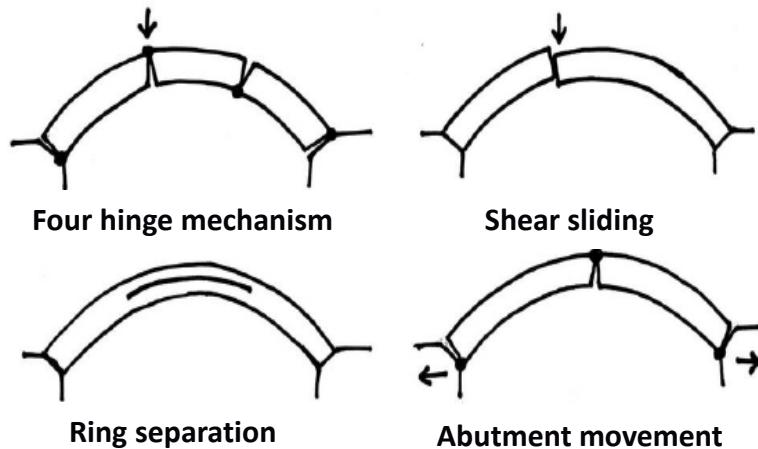
Computational strategies: masonry

Masonry material

Anisotropic response which depends upon

- characteristics of the individual components (bricks/blocks and mortar joints)
- masonry bond

Failure modes for masonry arches



Single ring arch



Multi-ring (stretcher bond)



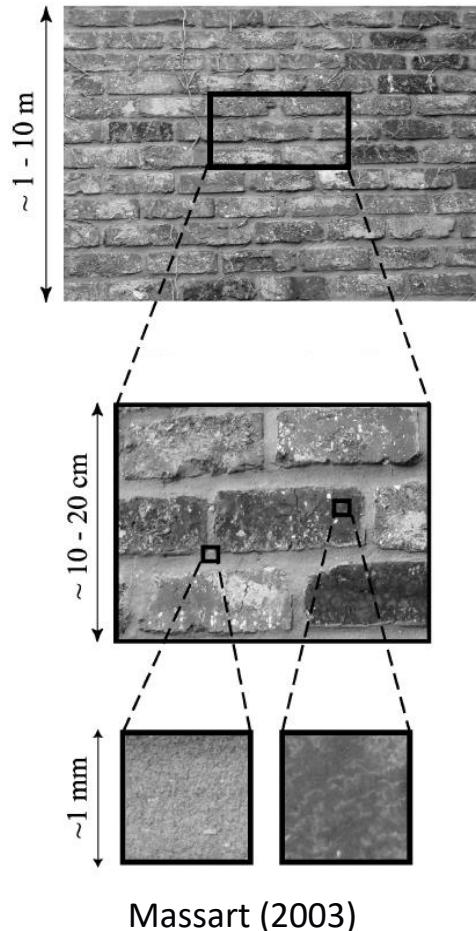
Multi-ring (header bond)





Computational strategies: masonry

Scale of representation



Structural
scale

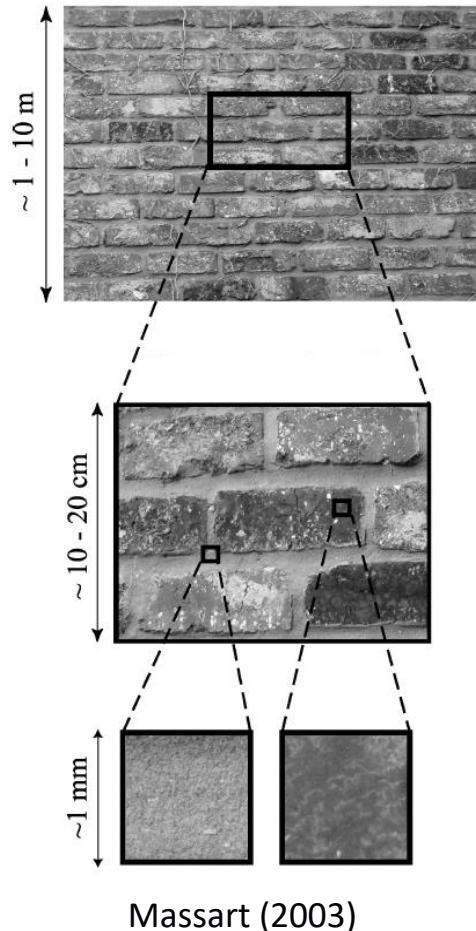
Mesoscale
scale

Microscale
scale



Computational strategies: masonry

Scale of representation



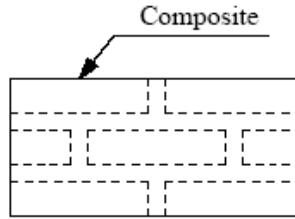
Structural
scale

Mesoscale
scale

Microscale
scale

Macroscale model

Equivalent material approach



Advantages

- Computational efficiency - analysis of large structures

Disadvantages

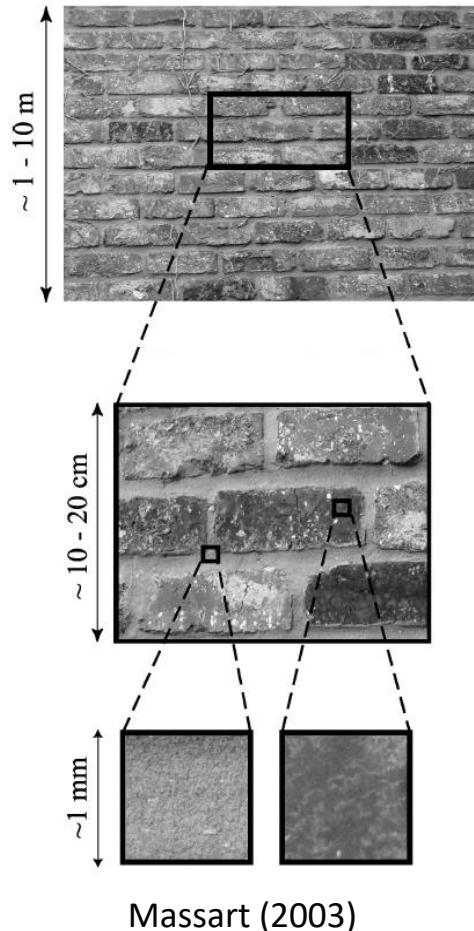
- Material model parameters must be obtained from tests on large components





Computational strategies: masonry

Scale of representation

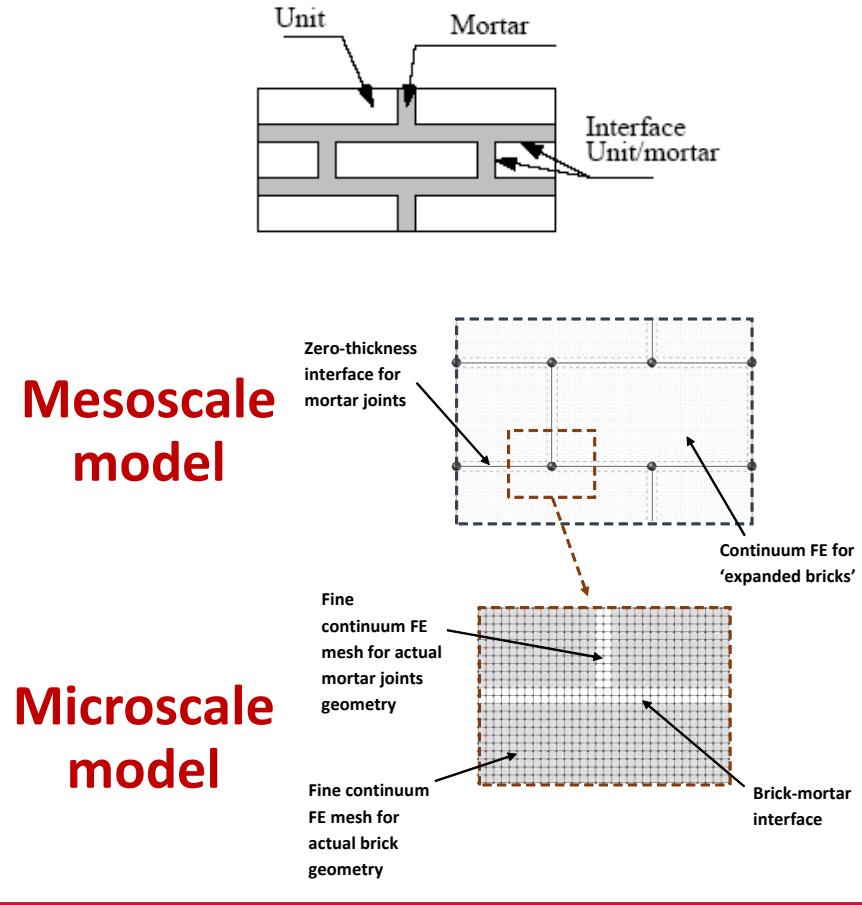


Structural
scale

Mesoscale
scale

Microscale
scale

Two-material approach



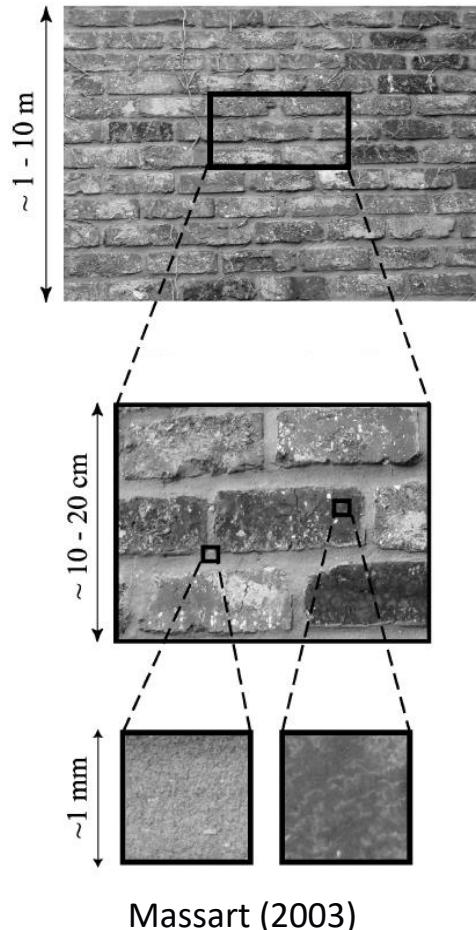
Mesoscale
model

Microscale
model



Computational strategies: masonry

Scale of representation



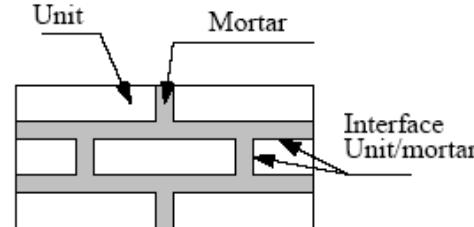
Structural
scale

Mesoscale
scale

Microscale
scale

Micro- Mesoscale models

Two-material approach



Advantages

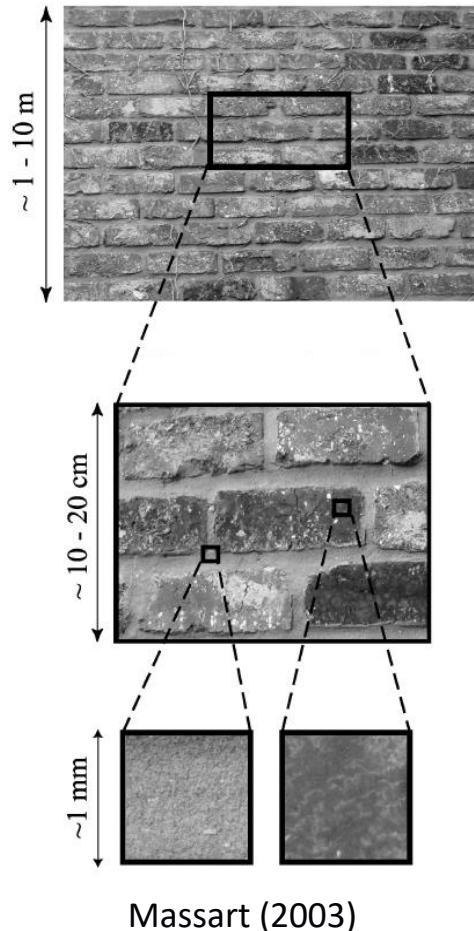
- Material parameters can be obtained by tests on constituents or small specimens





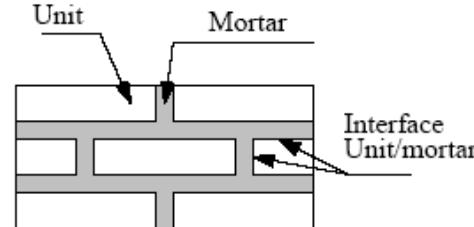
Computational strategies: masonry

Scale of representation



Micro- Mesoscale models

Two-material approach



Advantages

- Material parameters can be obtained by tests on constituents or small specimens

Disadvantages

- High computational cost - analysis of small components

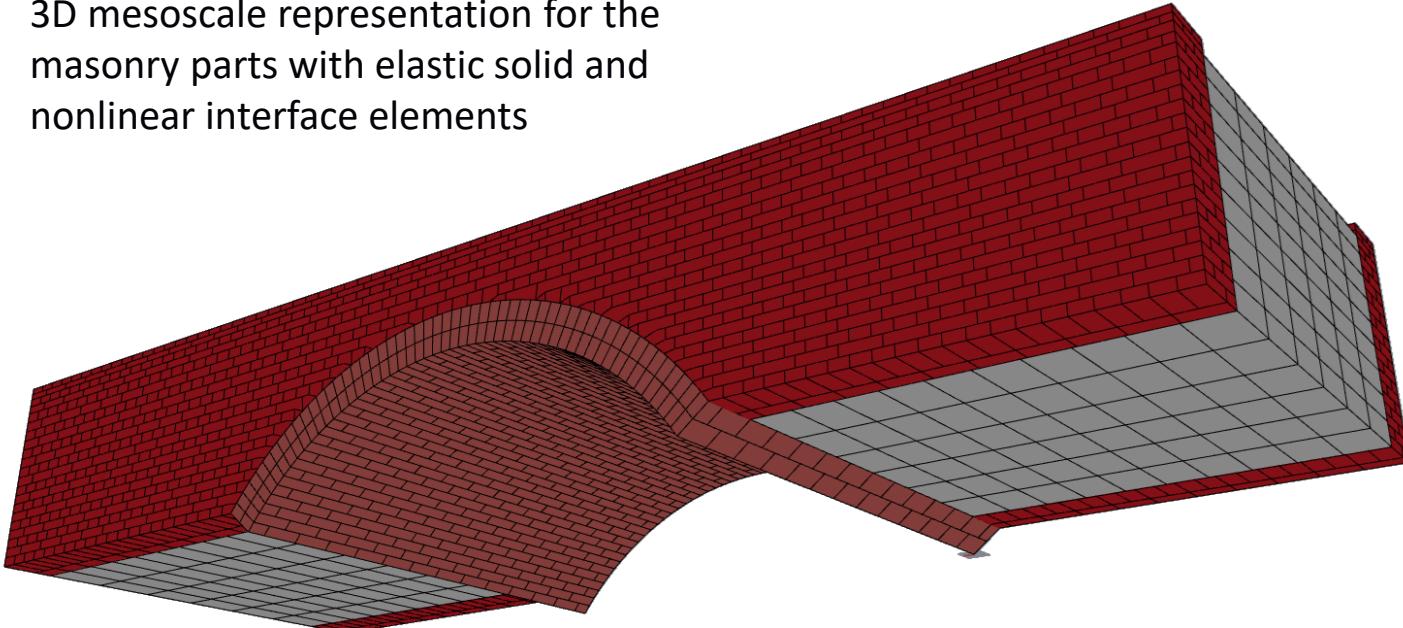


Computational strategies: bridges

Developments - CSM Group

3D mesoscale high-fidelity models

- 3D mesoscale representation for the masonry parts with elastic solid and nonlinear interface elements

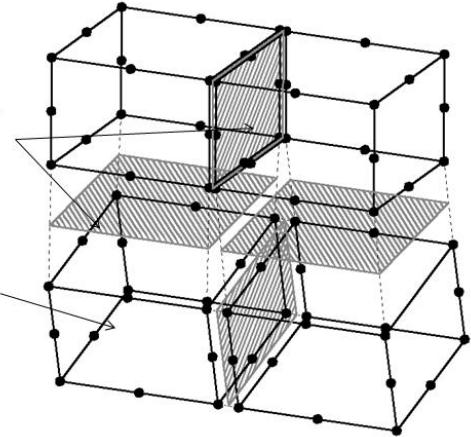


Detailed description of masonry bond for arch and spandrel walls

(Grosman et al. 2021)

mid-plane of 16-noded
interface element for
mortar joints

20-noded solid
element for bricks



- Elastoplastic material model for the backfill allowing for cohesion and friction
- Frictional cohesive nonlinear interface elements to represent the physical interfaces between the different bridge components

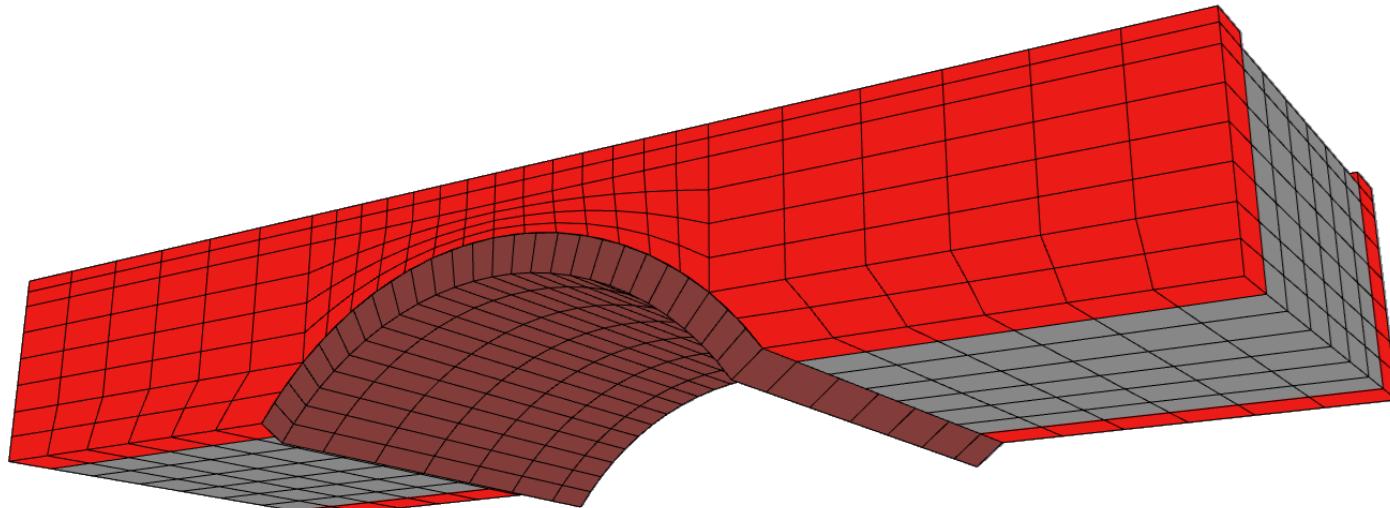


Computational strategies: bridges

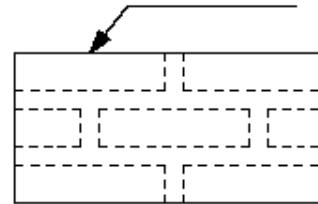
Developments - CSM Group

3D macroscale models

- Masonry components are modelled by an isotropic elasto-plastic material model allowing for damage (Lee & Fenves, 1998)



Masonry as a continuum material



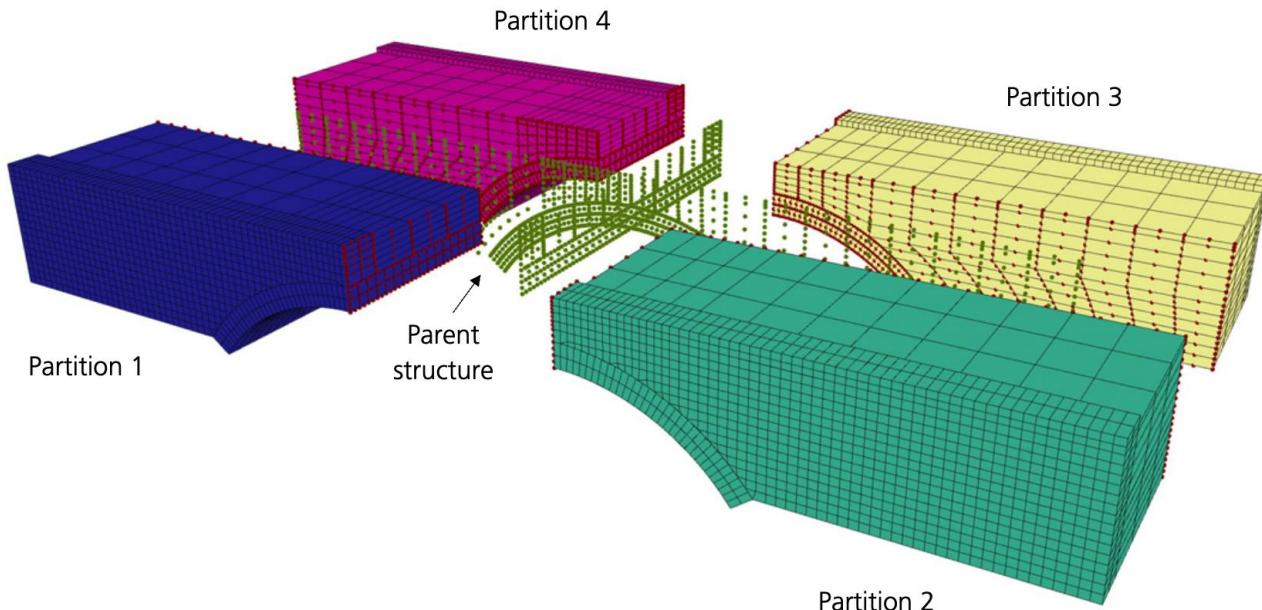
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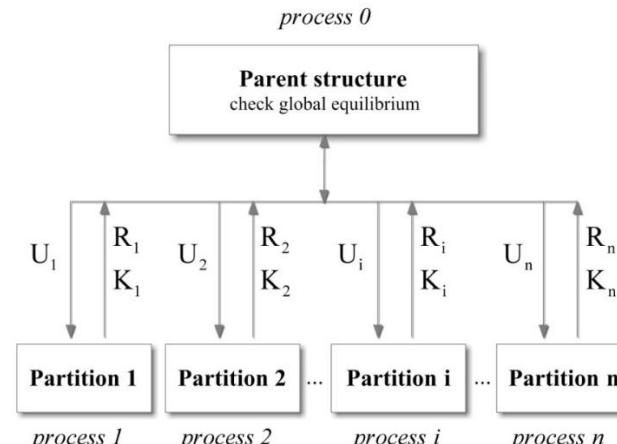
Computational strategies: bridges

Developments - CSM Group

Domain partitioning approach



- Parallel computation
- Improved computational efficiency



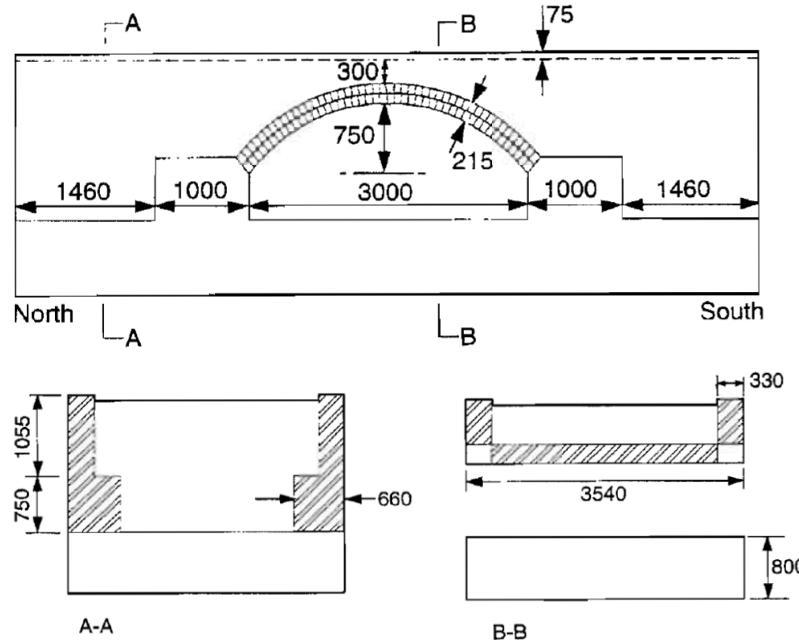
(Jokhio & Izzuddin 2015)

Parametric study details



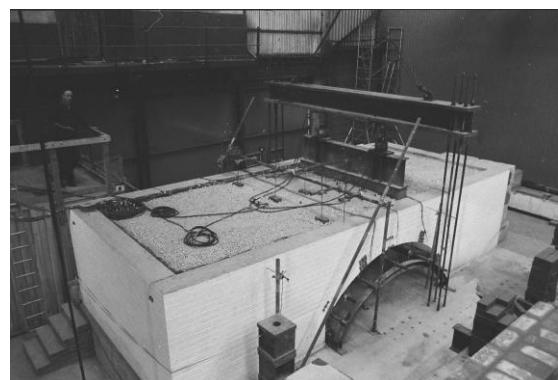
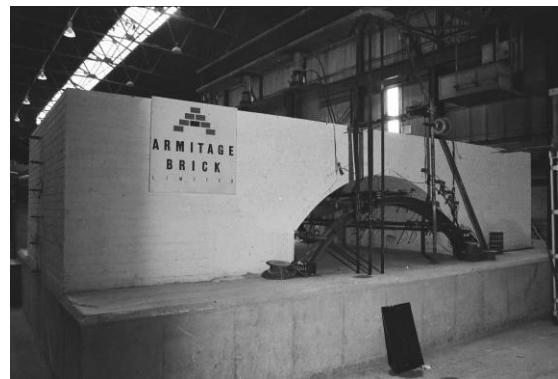
Parametric study details

Starting point: validated 3D mesoscale bridge models



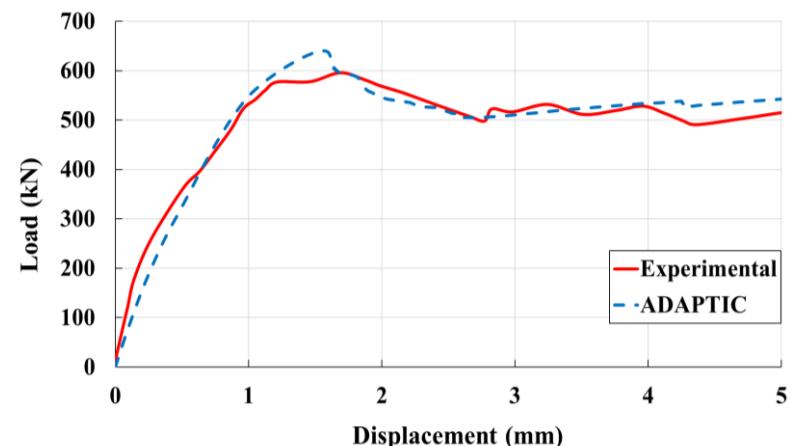
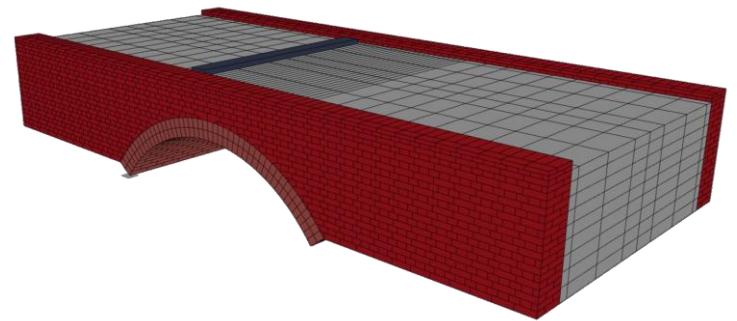
Bridge 3-3 Characteristics:

- Span = 3m
- Width = 3.54m
- Rise = 0.75m
- 2 Rings



Melbourne & Gilbert, 1995

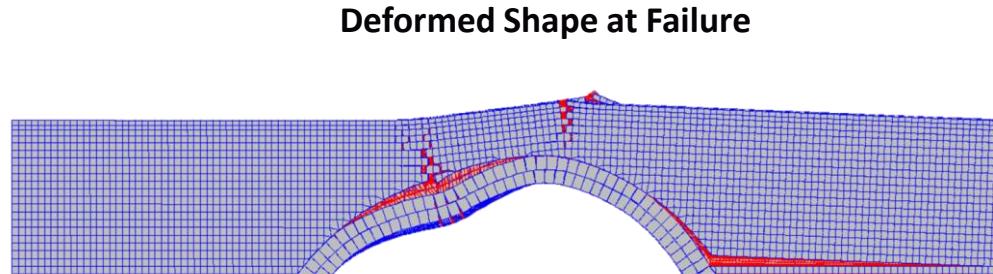
- Line load at quarter span increased up to collapse



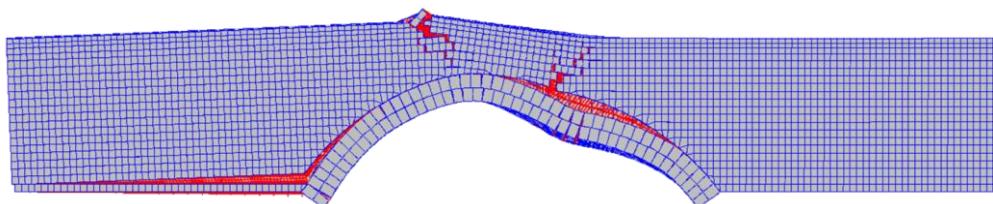


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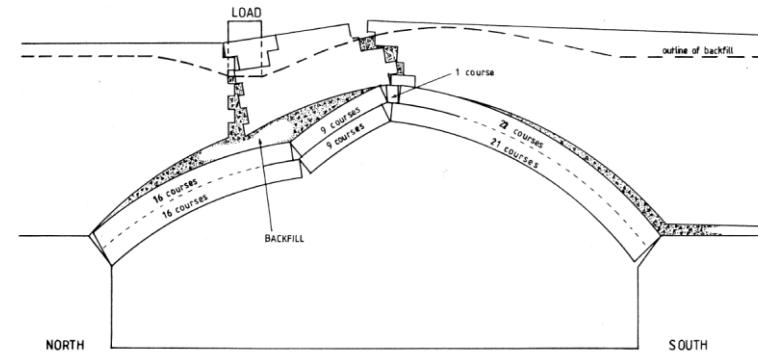
Starting point: validated 3D mesoscale bridge models



West Elevation



East Elevation

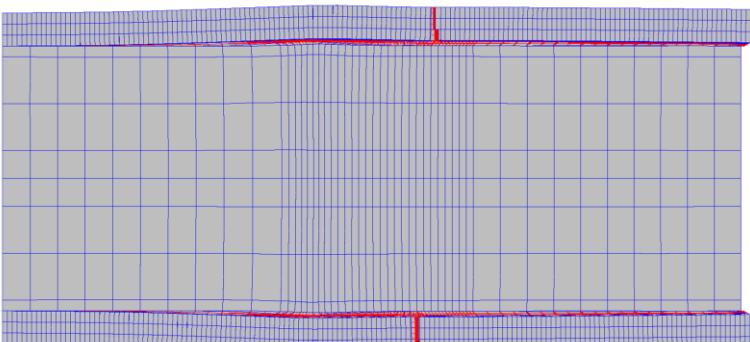
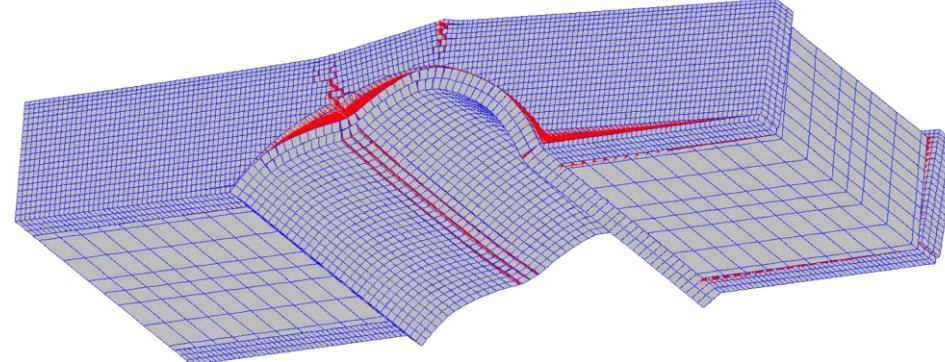
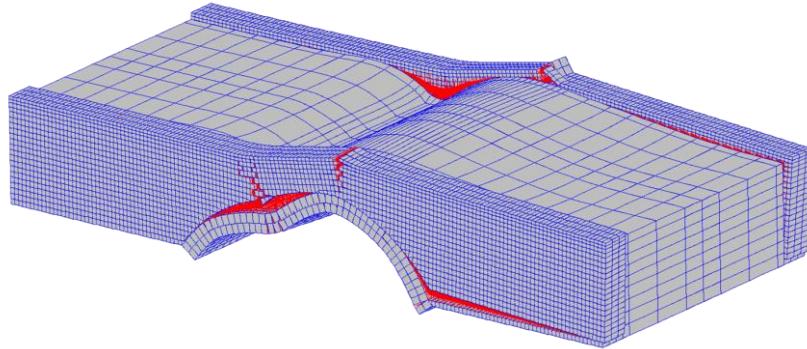


Melbourne & Gilbert, 1995



Parametric study details

Starting point: validated 3D mesoscale bridge models



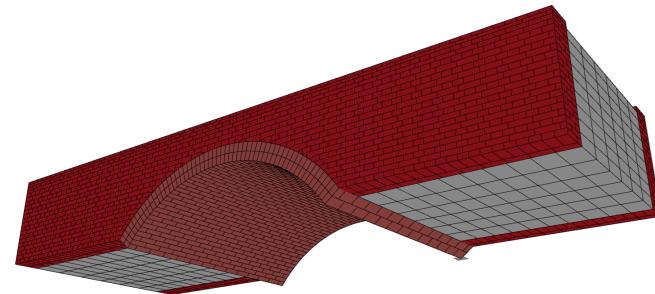
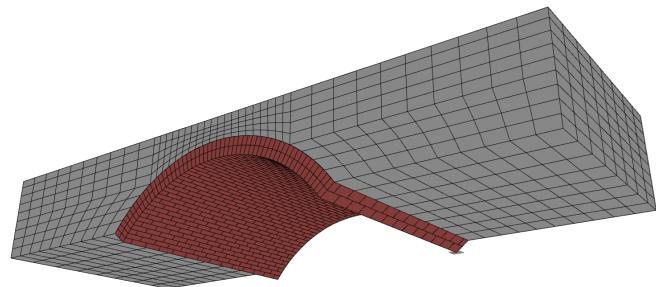
Melbourne & Gilbert, 1995



Parametric study details

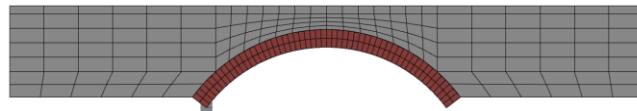
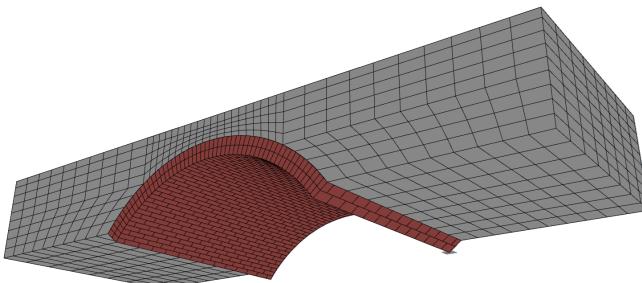
Validated high-fidelity models are used to:

- investigate 3D behaviour and interaction among different components



Comparing models with
attached and detached
spandrel walls

- establish realistic effective width values for 2D simulation



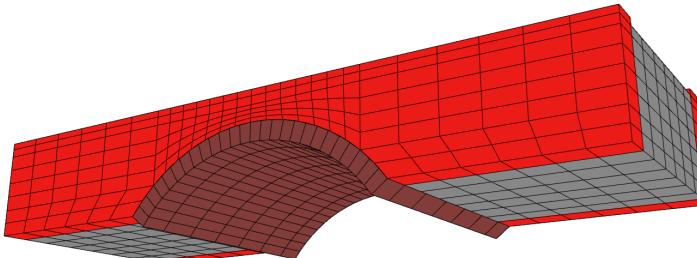
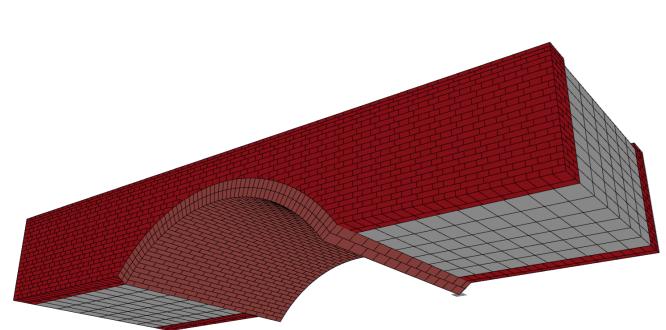
Comparing predictions
from 2D and 3D models



Parametric study details

Validated high-fidelity models are used to

- **assess accuracy of reduced FE models**



Compare meso against
macroscale models

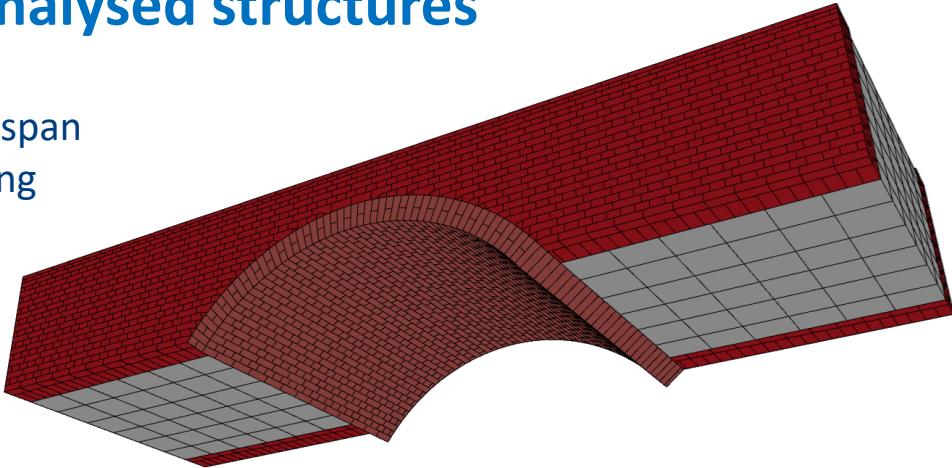
- **obtain baseline response data for multi-level assessment**



Parametric study details

Analysed structures

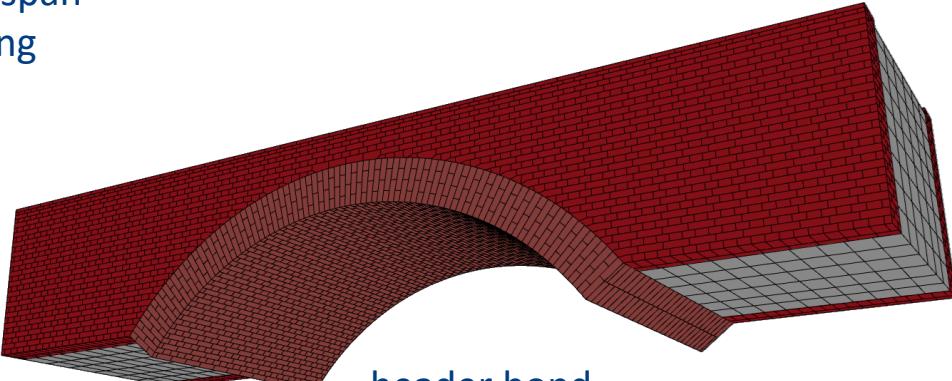
3m span
2-ring



header bond

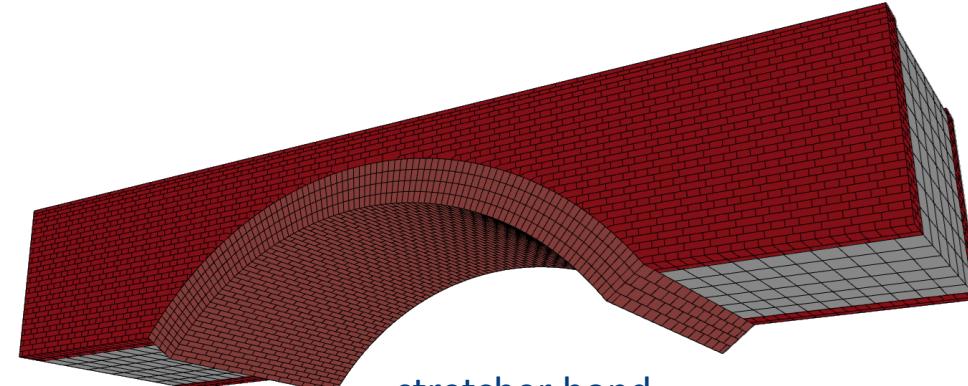
Width 3m, 6m, 9m
Rise-to-span 1:4

5m span
4-ring



header bond

stretcher bond



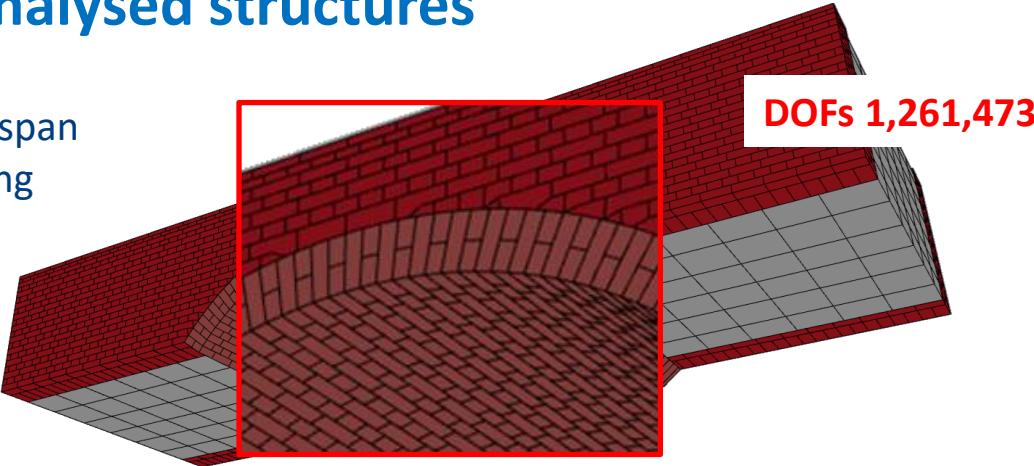
stretcher bond



Parametric study details

Analysed structures

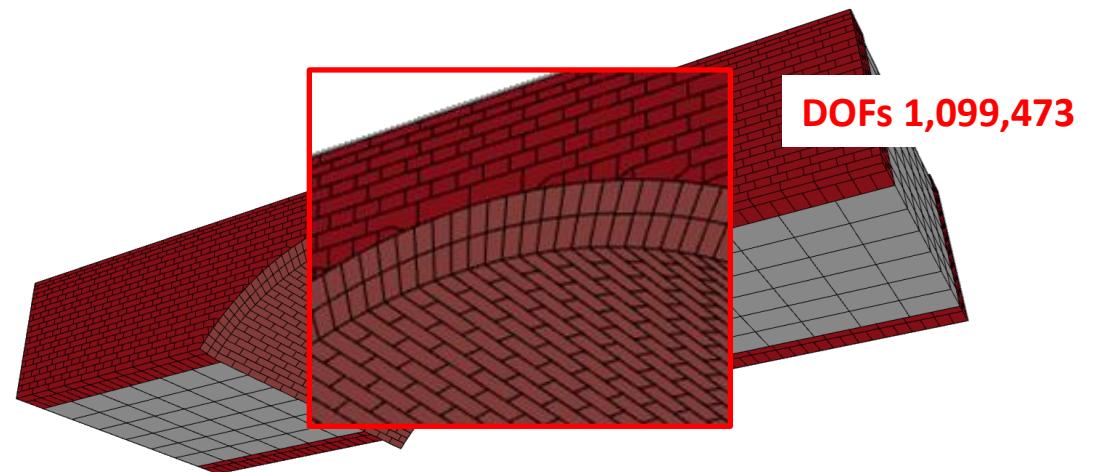
3m span
2-ring



header bond

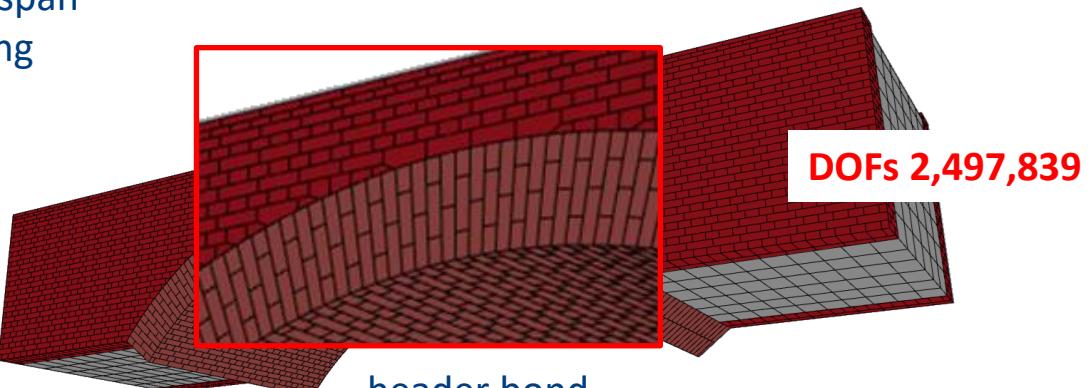
DOFs 1,261,473

Width **3m**, 6m, 9m
Rise-to-span 1:4



stretcher bond

5m span
4-ring



header bond

DOFs 2,497,839



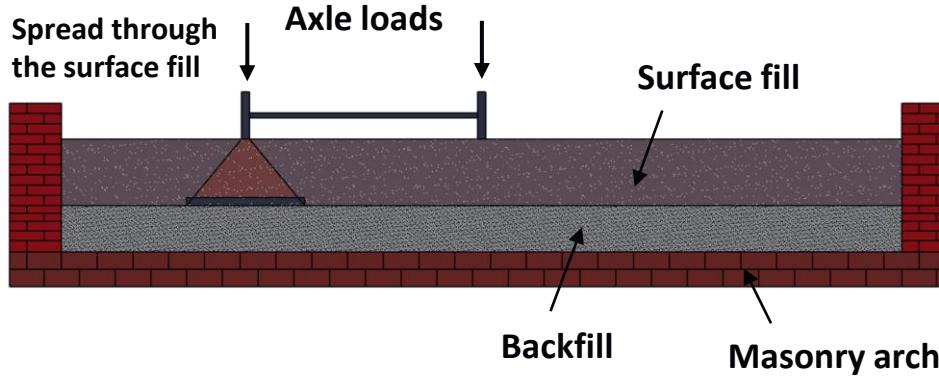
stretcher bond

DOFs 1,957,839

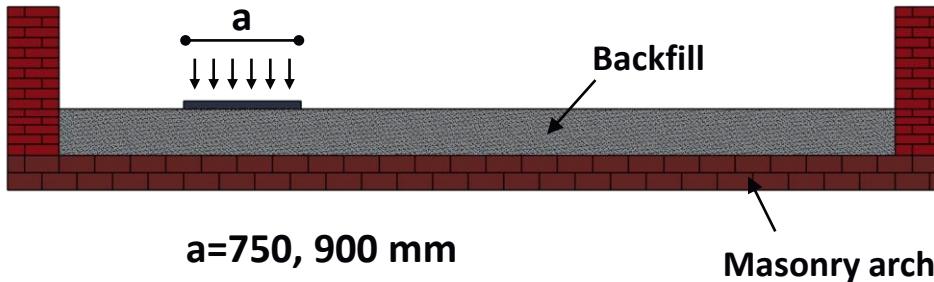


Parametric study details

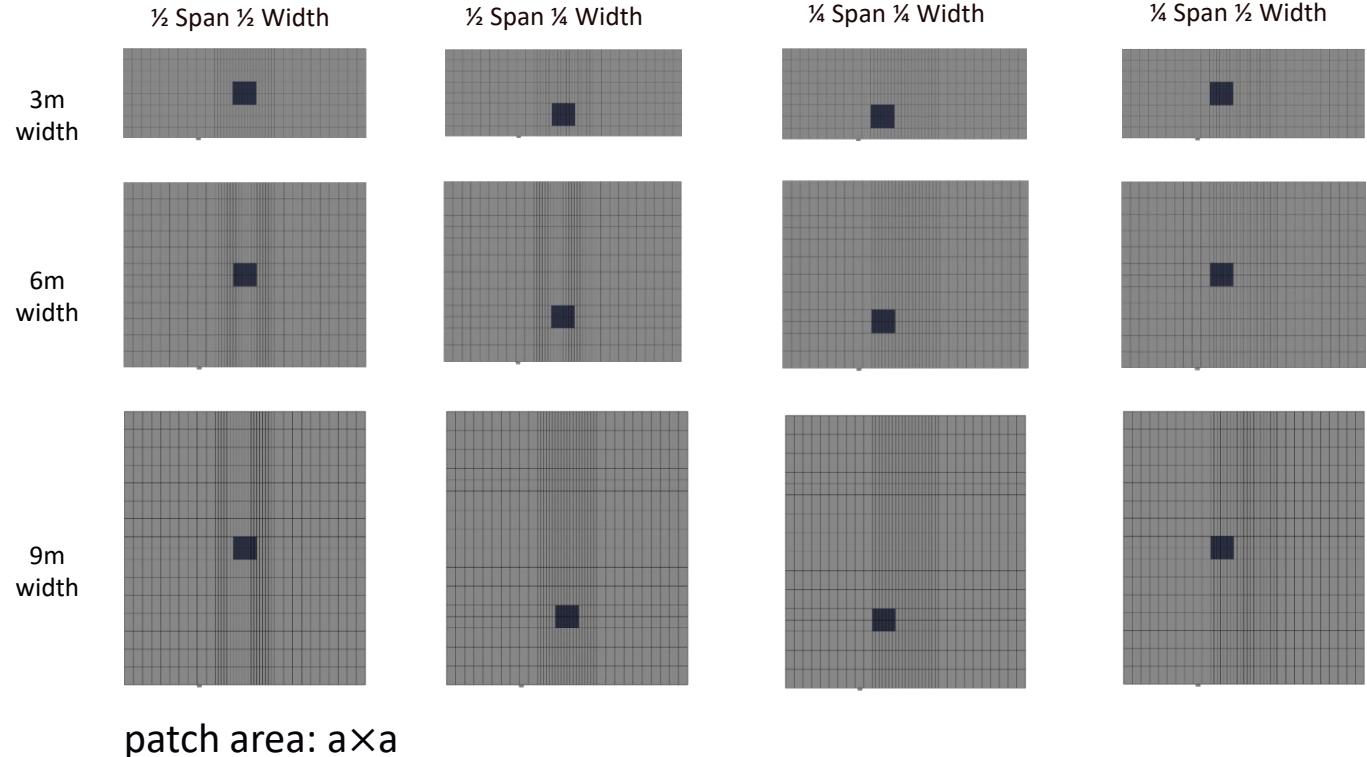
Loading



Patch load on the backfill



Patch load positions





Parametric study details

Material parameters for units and mortar joints*

Brick units		
E (Young's modulus)	N/mm ²	69000
v (Poison's ratio)		0.15
Mortar Interfaces		
K _n (normal stiffness)	N/mm ³	295
K _t (tangent stiffness)	N/mm ³	118
F _t (tensile strength)	N/mm ²	0.21
tan(φ)(friction angle)	rad	0.64
c (cohesion)	N/mm ²	0.29
F _c (compressive strength)	N/mm ²	23.8
G _{f_t} (Fracture energy in tension)	Nmm/mm ²	0.01
G _{f_s} (Fracture energy shear)	Nmm/mm ²	0.029

Material parameters for backfill*

Backfill		
E _f (Young's modulus)	N/mm ²	200
v (Poison's ratio)		0.2
c (cohesion)	N/mm ²	0.001
φ _f (friction angle)	rad	1.047
Ψ _f (dilatancy angle)	rad	0.5236

*from material tests (Melbourne & Gilbert 1995) used for model validation

Numerical results

Spandrel wall study

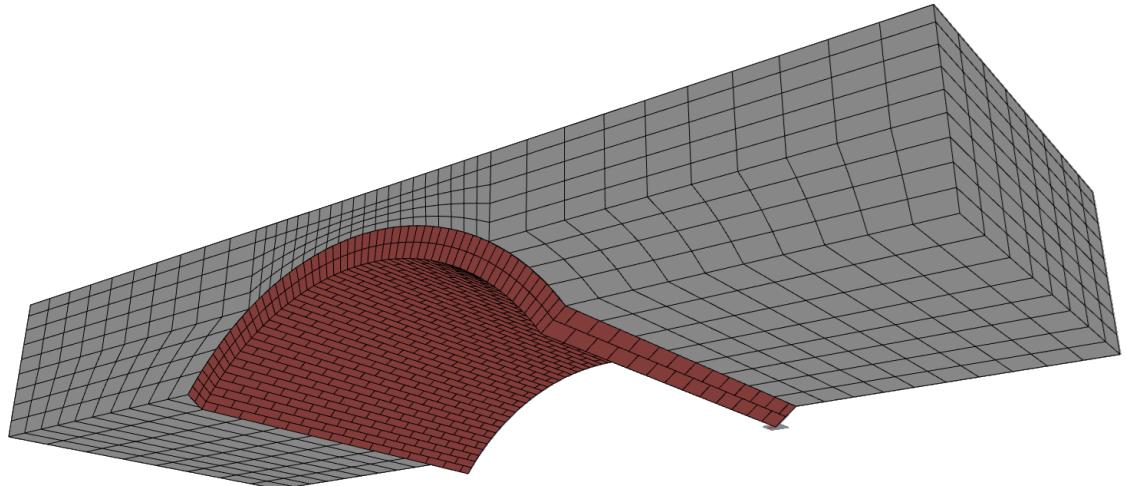
Numerical results

Influence of spandrel walls

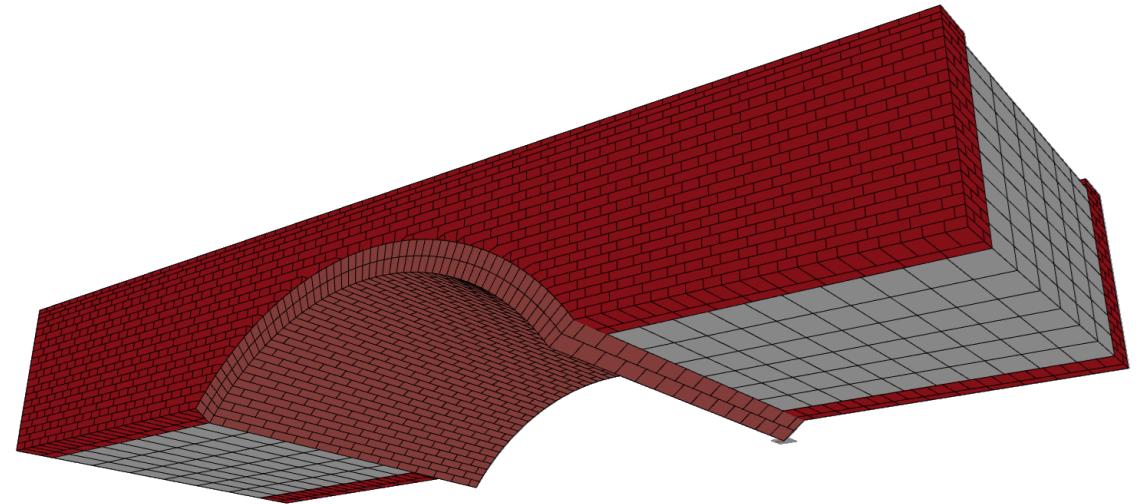


Detached vs Attached Spandrel Wall models

- 3m Span
- 3m Width
- 0.75m Rise
- 2 Rings
- SW thickness 330mm
- Stretcher Bond



Detached SW



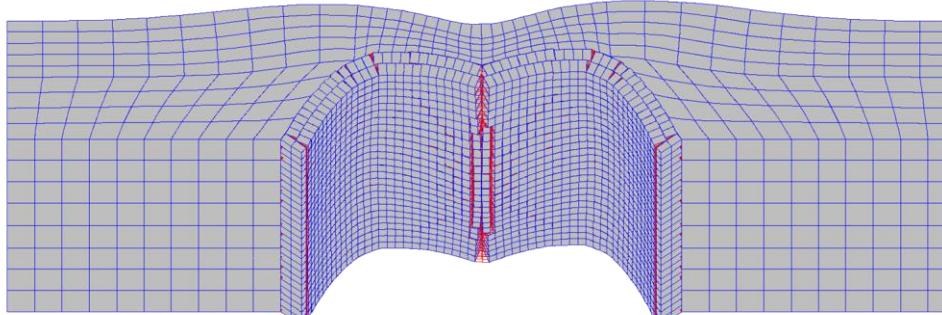
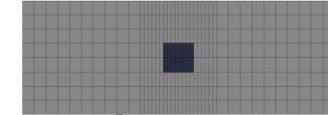
Attached SW

Numerical results

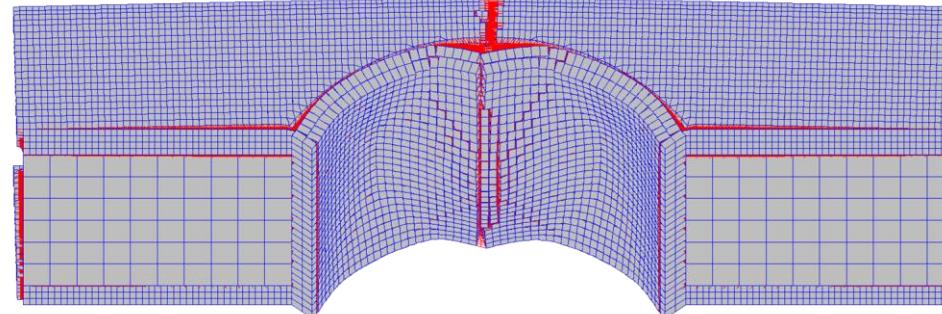
Influence of spandrel walls



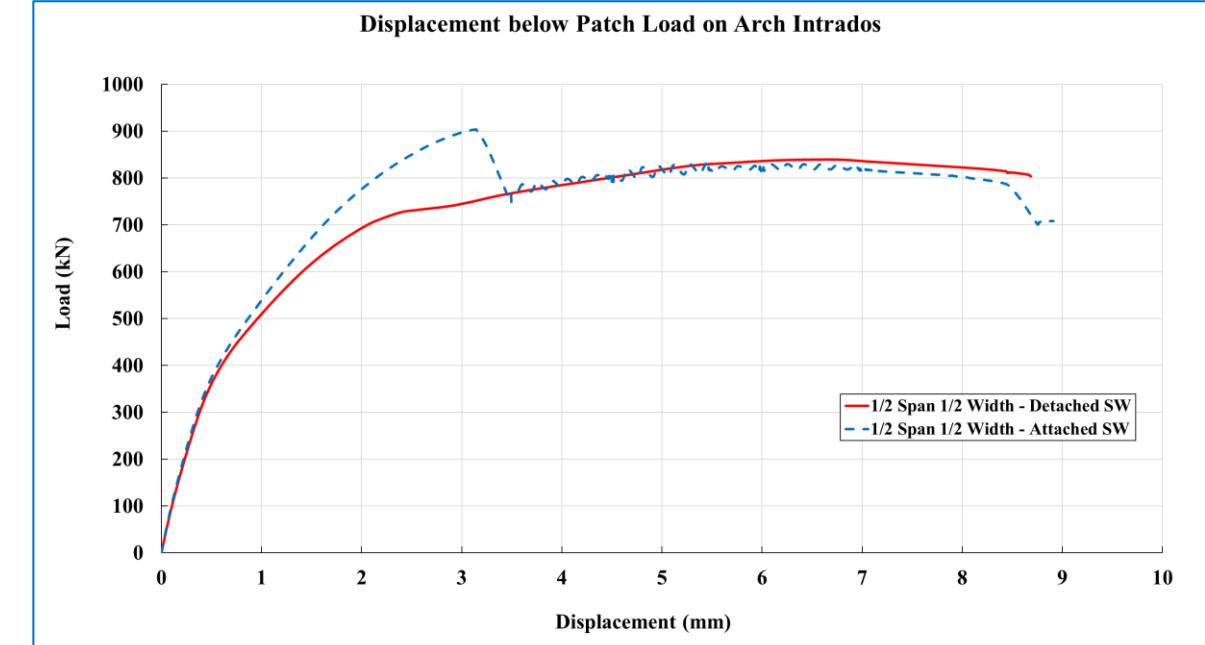
Patch 750mm×750mm at $\frac{1}{2}$ Span $\frac{1}{2}$ Width



Detached SW



Attached SW – West Elevation

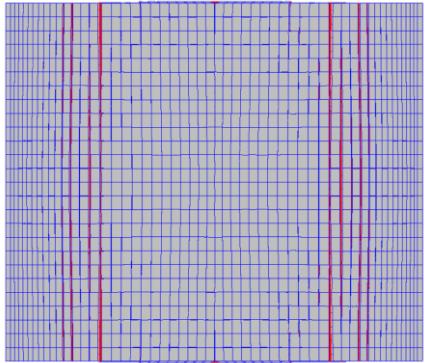


Numerical results

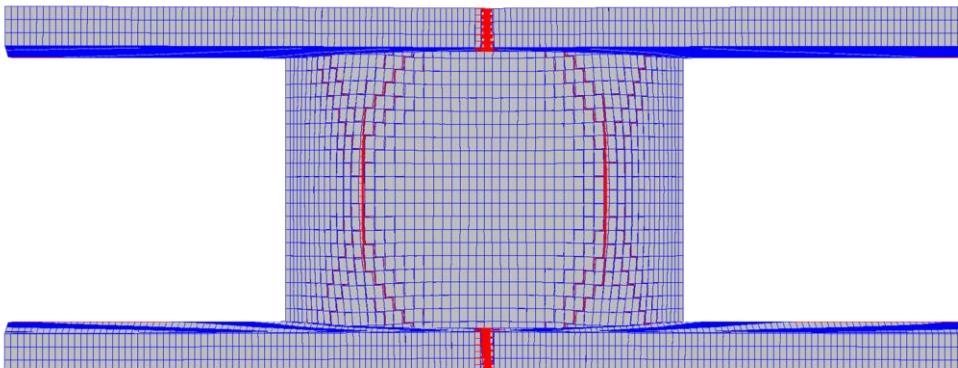
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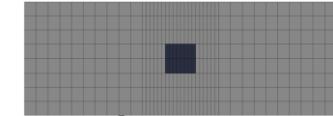
Patch 750mm×750mm at $\frac{1}{2}$ Span $\frac{1}{2}$ Width



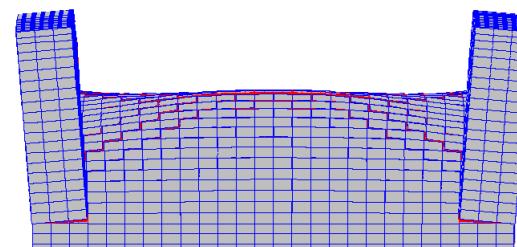
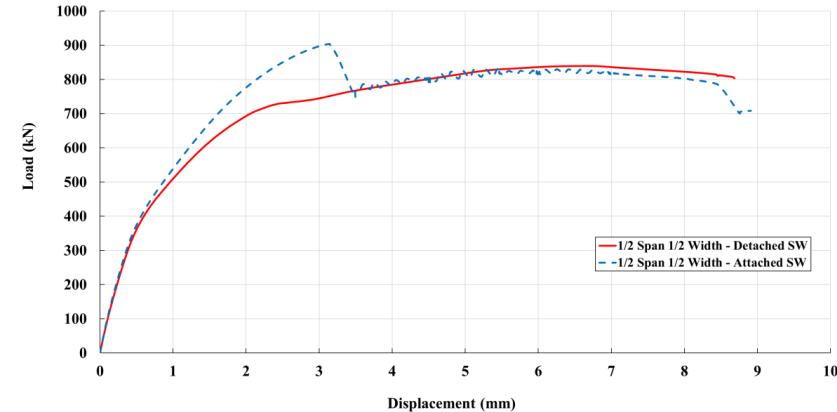
Detached SW – Top View



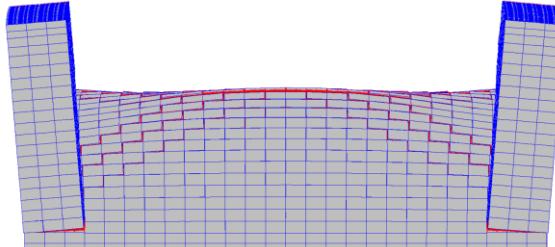
Attached SW – Top View



Displacement below Patch Load on Arch Intrados



Attached SW – Right Side View



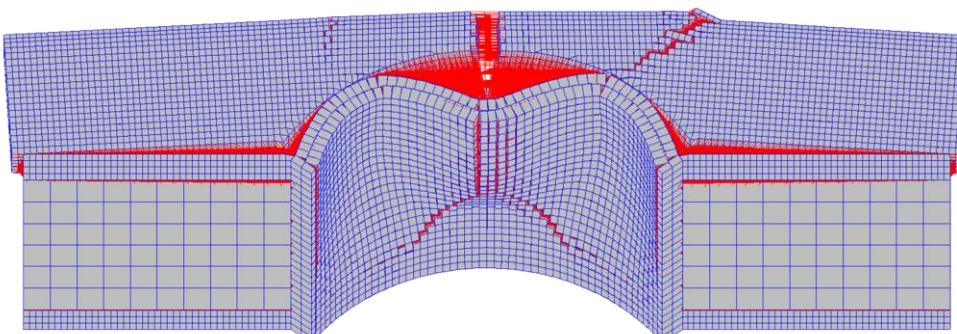
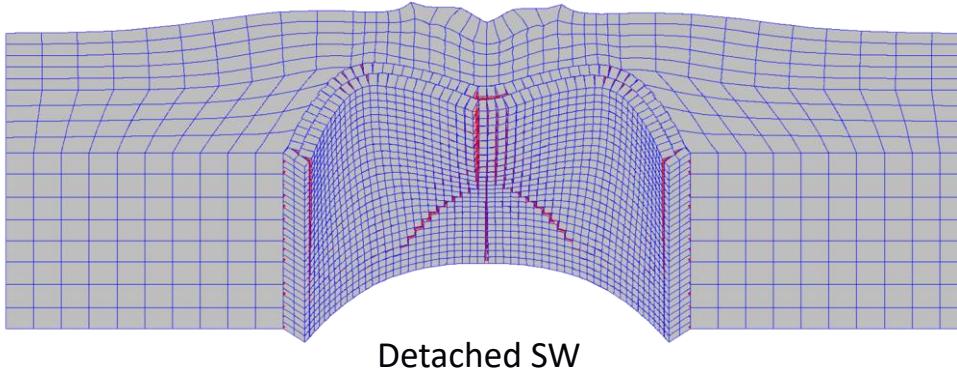
Attached SW – Left Side View

Numerical results

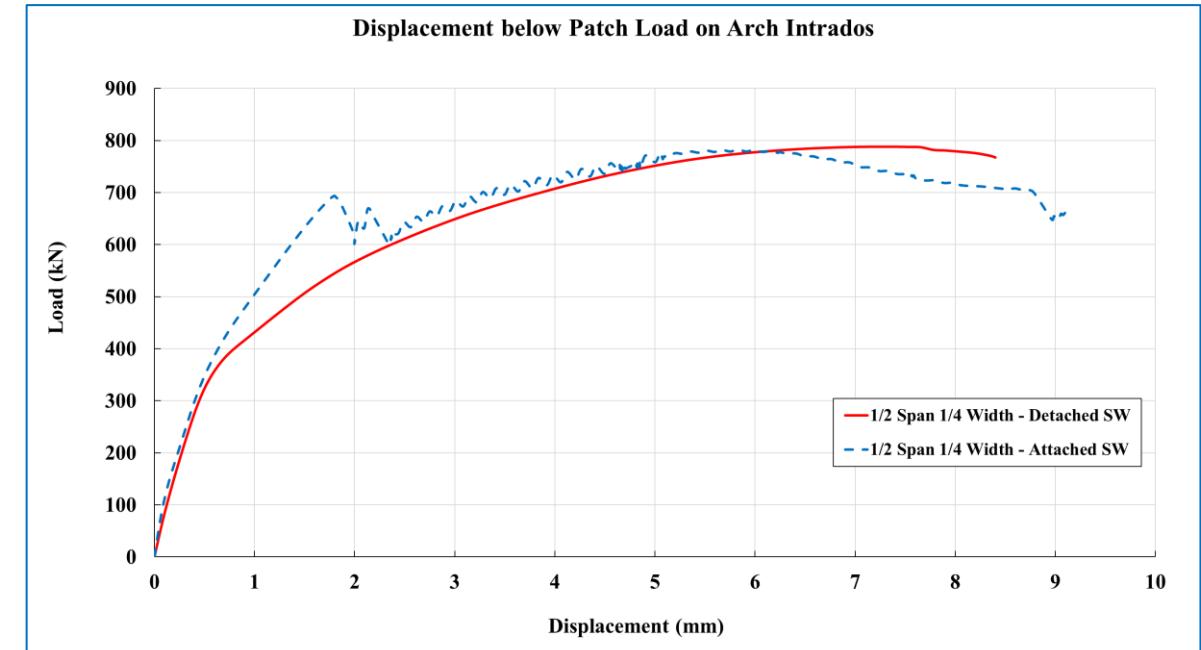
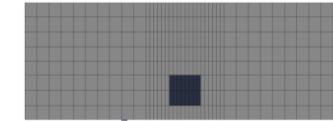
Influence of spandrel walls



Patch 750mm×750mm at $\frac{1}{2}$ Span $\frac{1}{4}$ Width



Attached SW – West Elevation

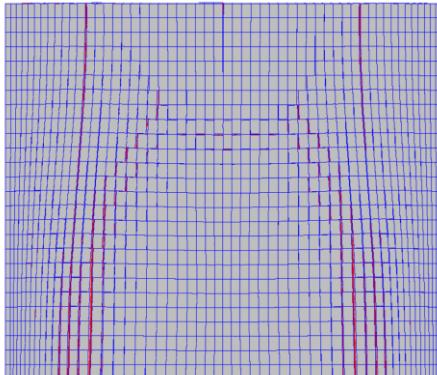


Numerical results

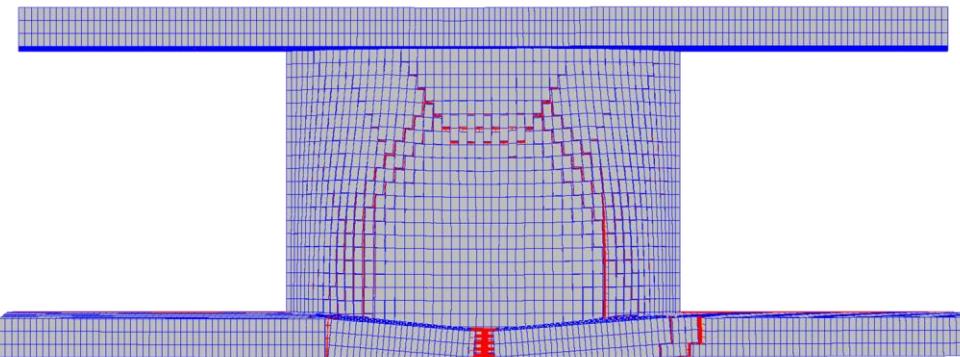
Influence of spandrel walls



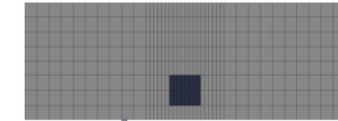
Patch 750mm×750mm at $\frac{1}{2}$ Span $\frac{1}{4}$ Width



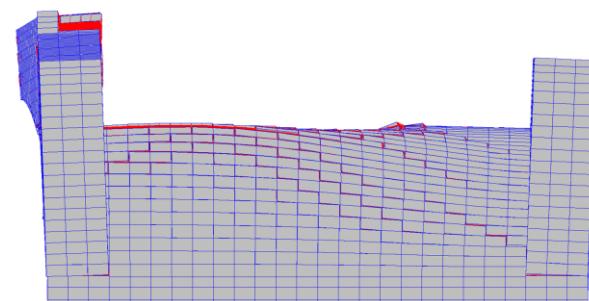
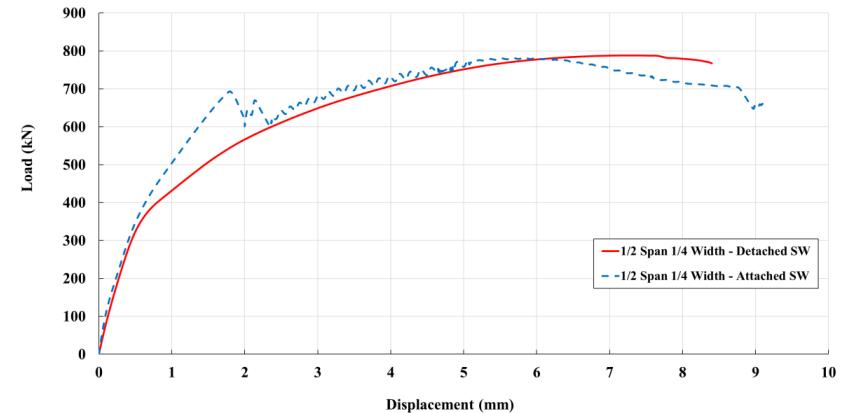
Detached SW – Top View



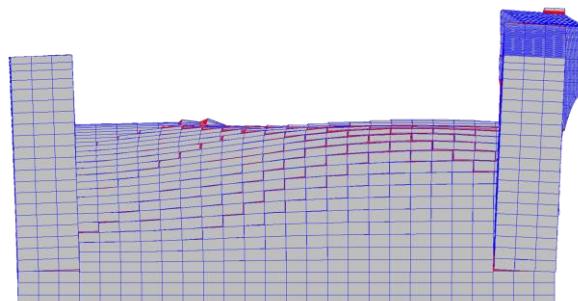
Attached SW – Top View



Displacement below Patch Load on Arch Intrados



Attached SW – Right Side View



Attached SW – Left Side View

Numerical results

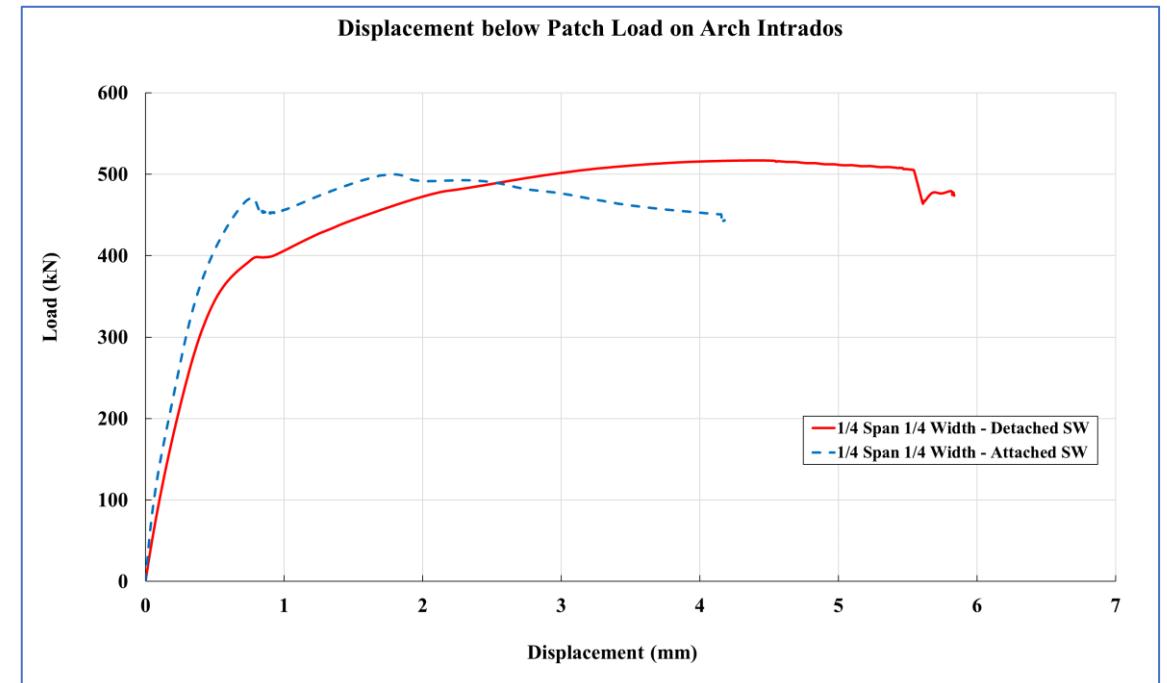
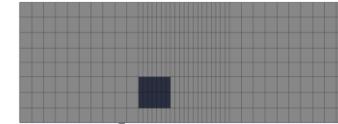
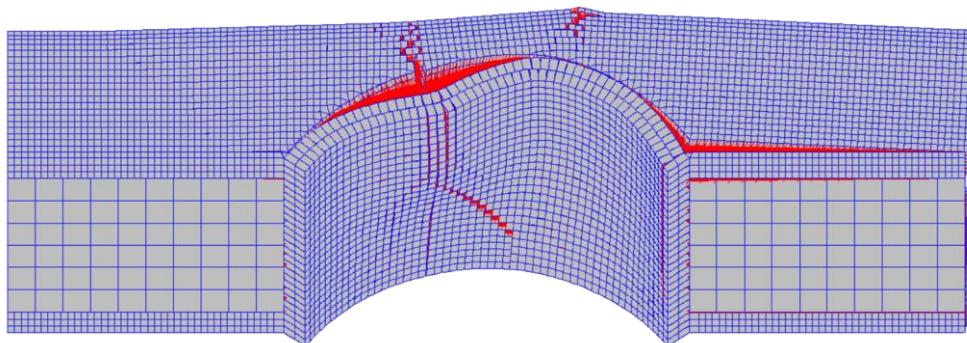
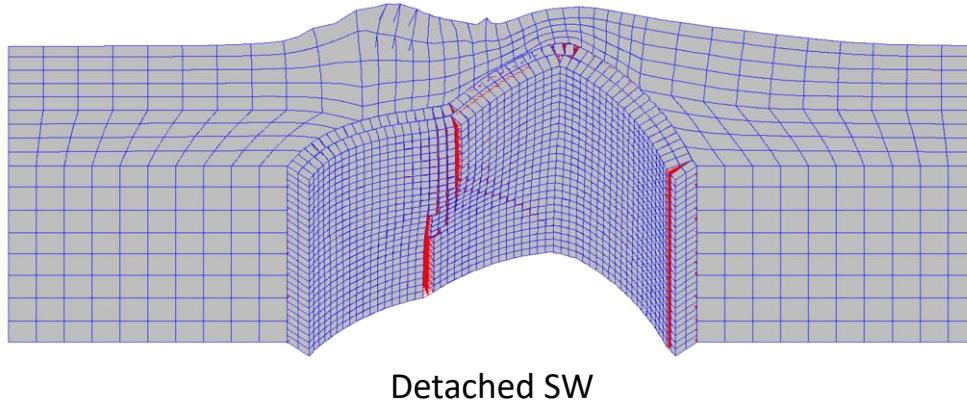
Influence of spandrel walls



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Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{4}$ Width

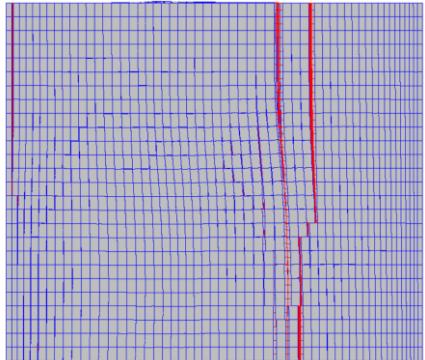


Numerical results

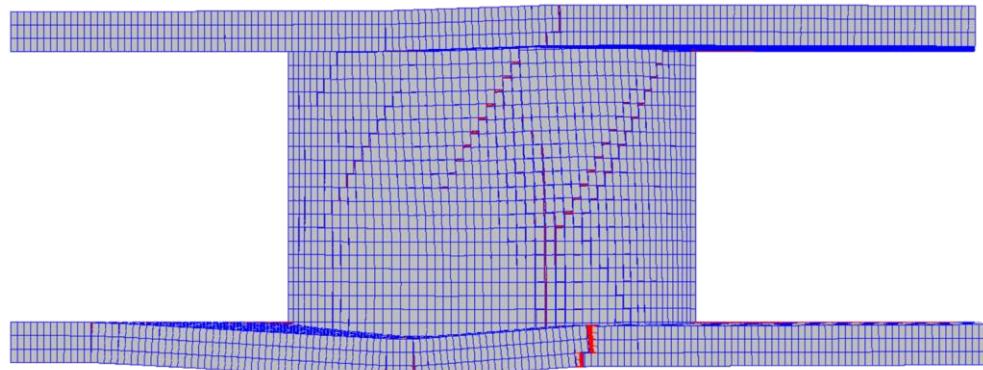
Influence of spandrel walls



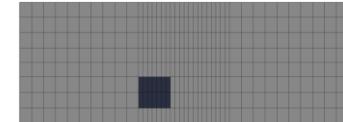
Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{4}$ Width



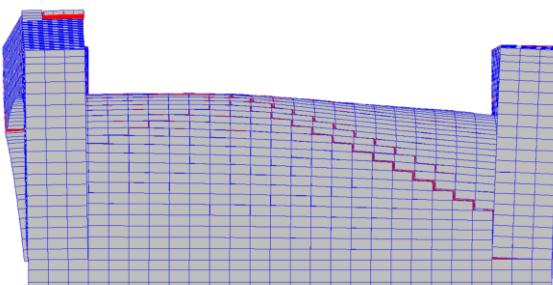
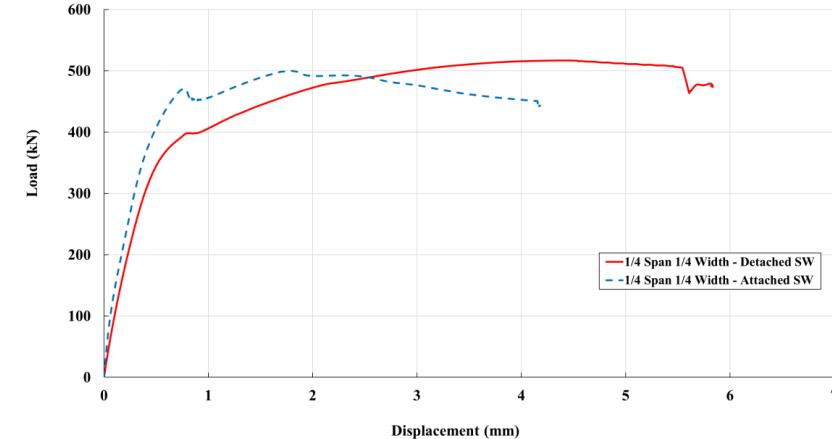
Detached SW – Top View



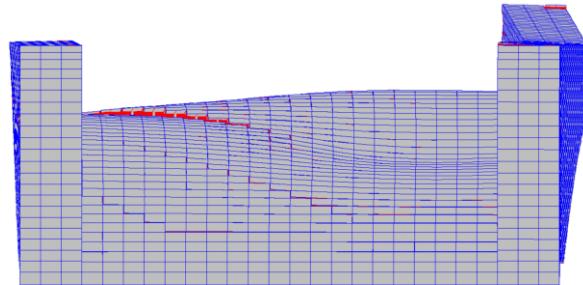
Attached SW – Top View



Displacement below Patch Load on Arch Intrados



Attached SW – Right Side View

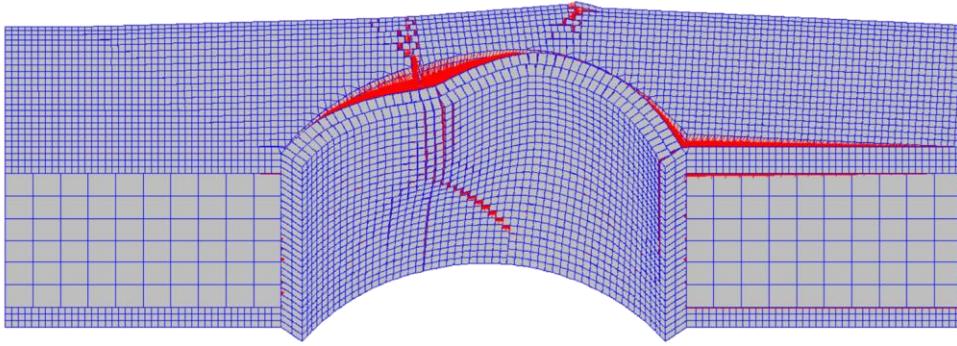


Attached SW – Left Side View

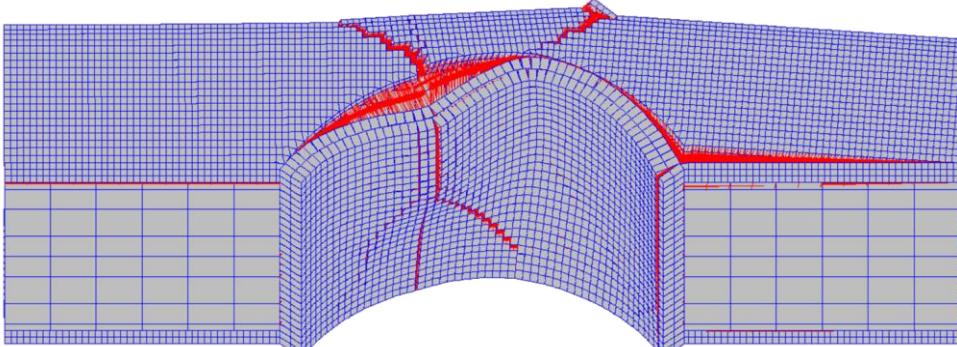
Numerical results

Influence of spandrel walls

SW thickness



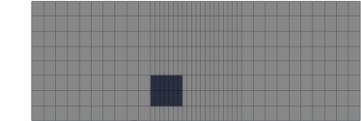
SW 330mm – West Elevation



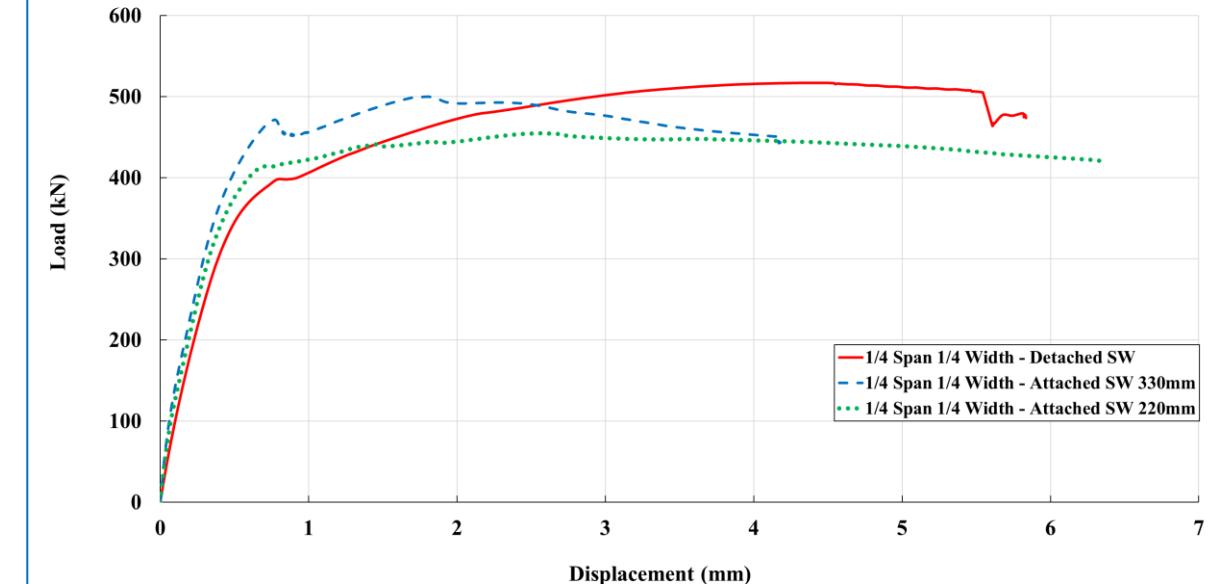
SW 220mm – West Elevation



Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{4}$ Width



Displacement below Patch Load on Arch Intrados

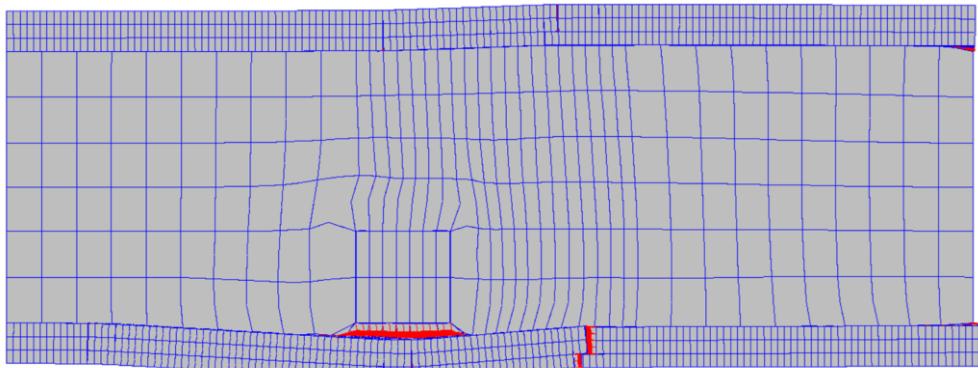


Numerical results

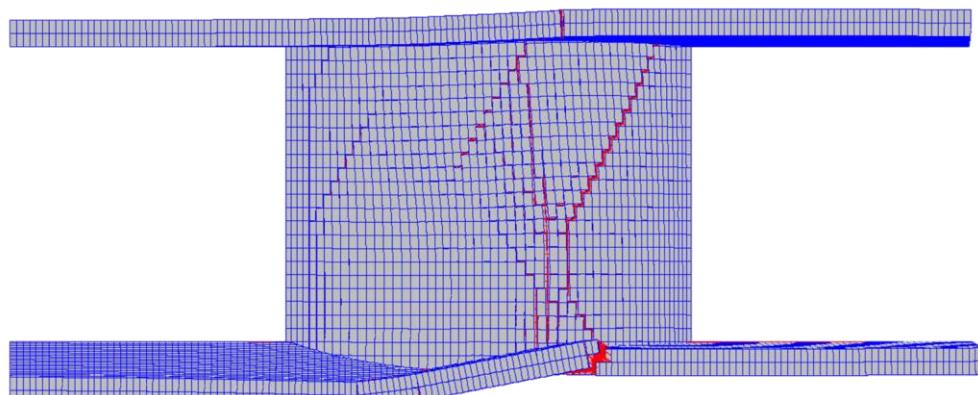
Influence of spandrel walls



SW thickness

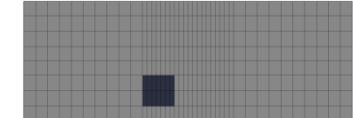


SW 330mm – Top View

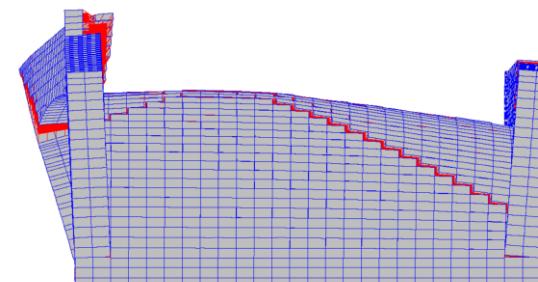
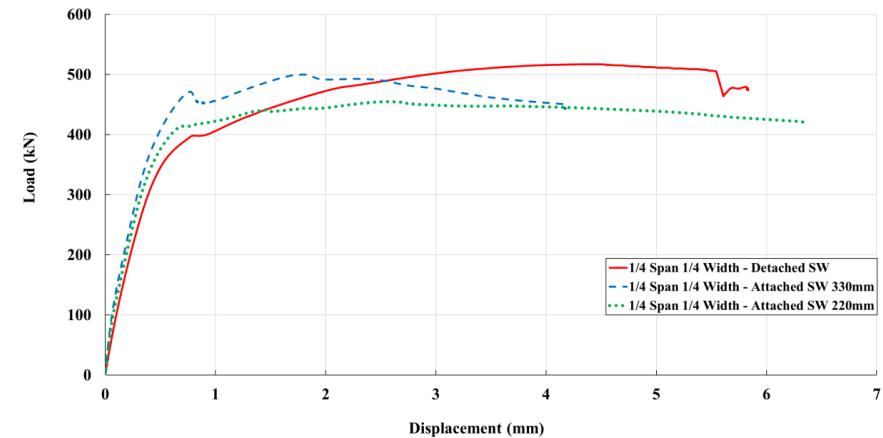


SW 220mm – Top View

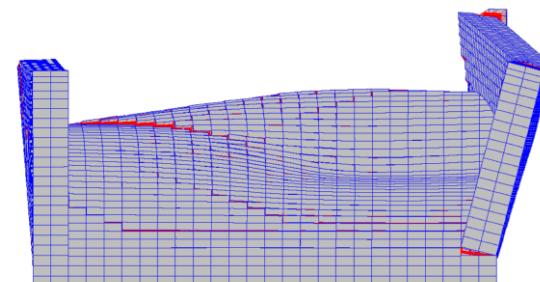
Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{4}$ Width



Displacement below Patch Load on Arch Intrados



SW 220mm – Right Side View



SW 220mm – Left Side View

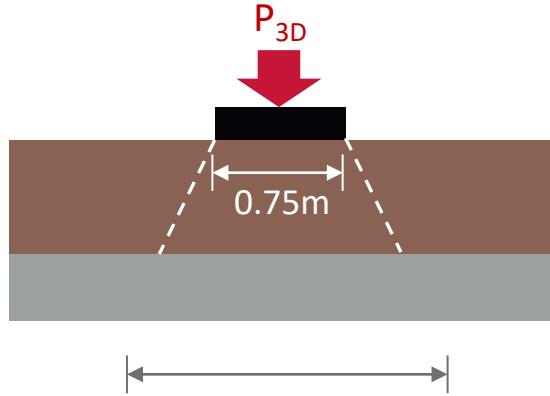
Effective width study

Numerical results

3D vs 2D models – effective width

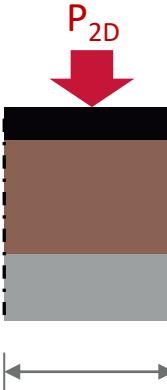


3D model:



Effective width = ?

2D model:



Width = 1m

$$\text{Effective width (single wheel)} = \frac{P_{3D}}{P_{2D}} \times 1\text{m}$$

Numerical results

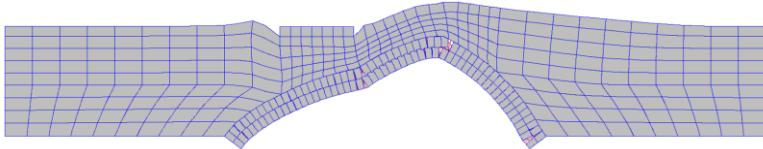
3D vs 2D models – effective width

Effective width depends on

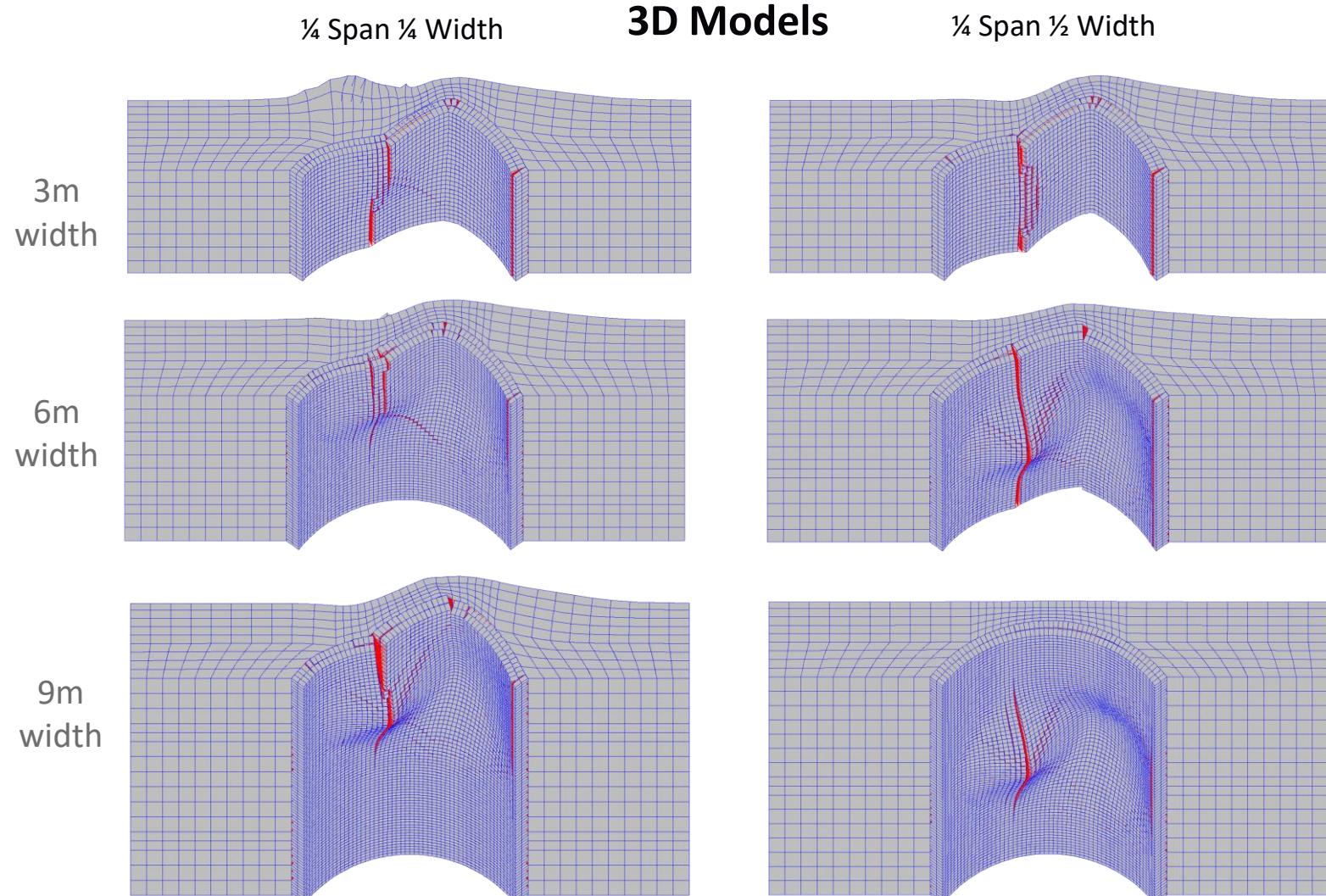
- Load position
- Backfill characteristics
- 3D arch behaviour

2D Model

$\frac{1}{4}$ Span



3m span, stretcher bond

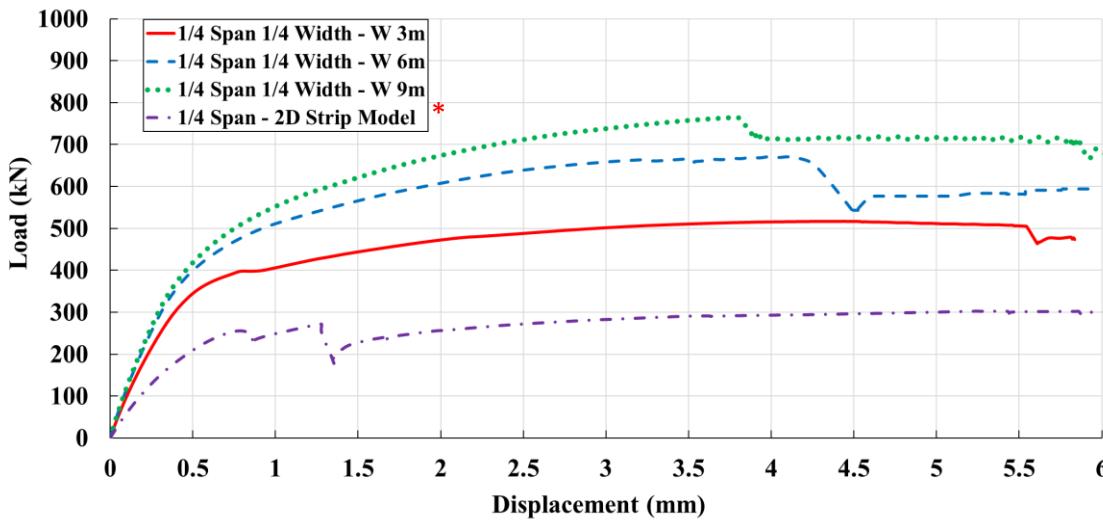


Numerical results

3D vs 2D models – effective width



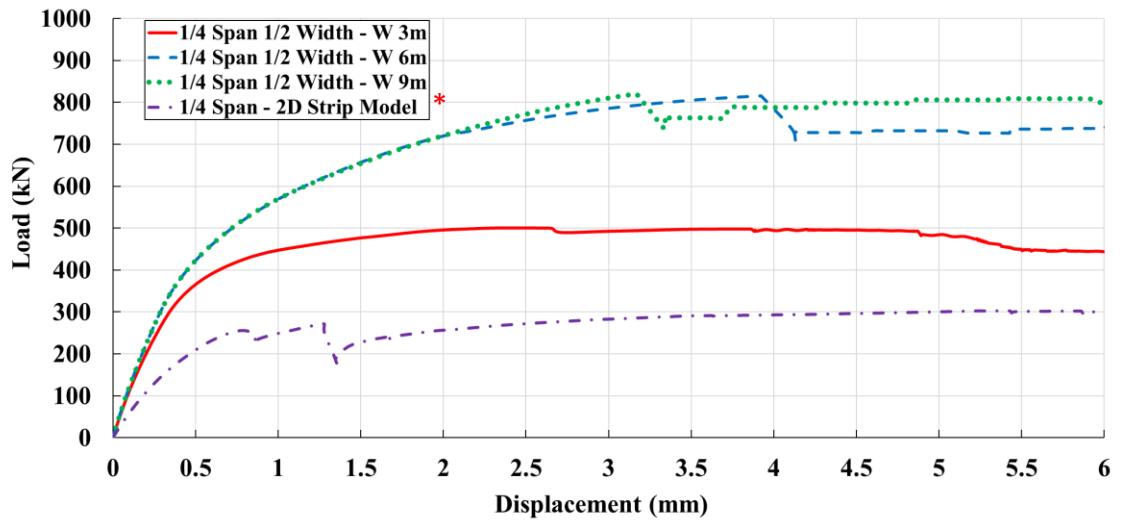
Displacement below Patch Load on Arch Intrados



*Load predicted by 2D model assuming 1m width



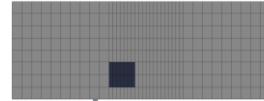
Displacement below Patch Load on Arch Intrados



- Higher load capacity for 6m and 9m widths (3D response of masonry arch)

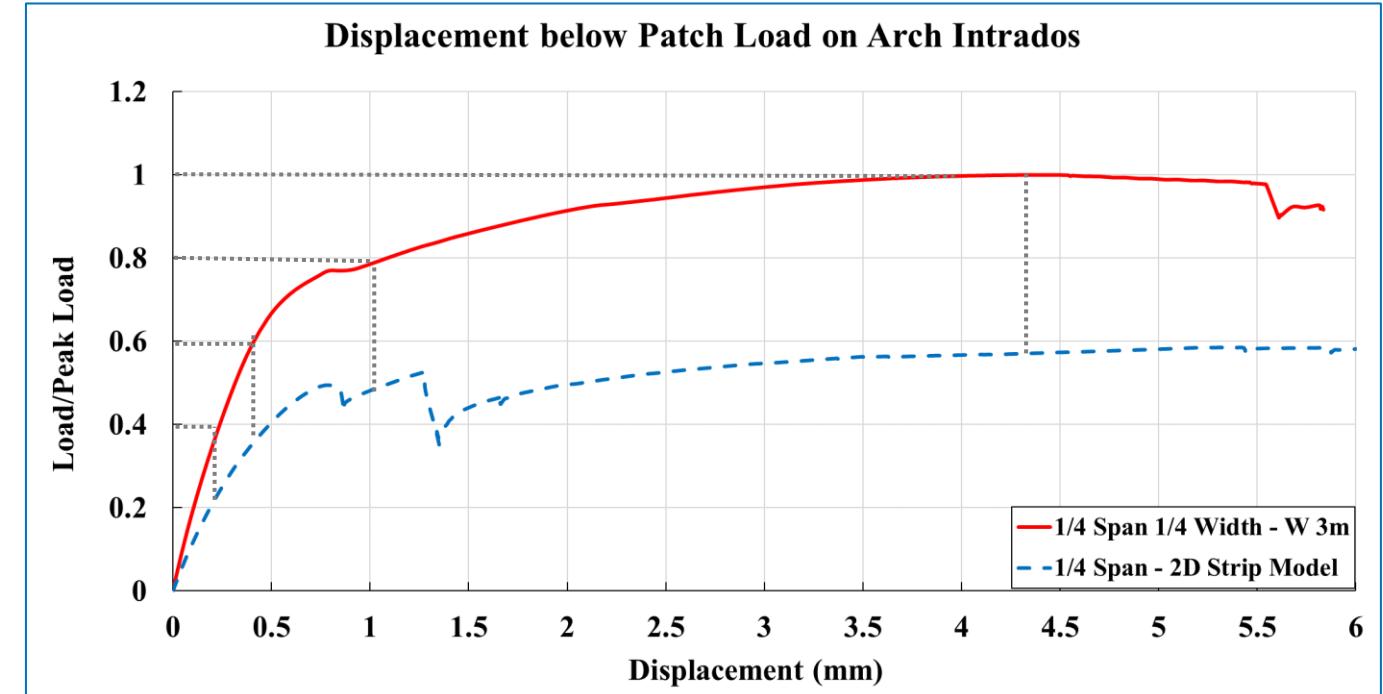
Numerical results

3D vs 2D models – effective width



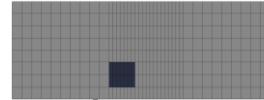
Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{4}$ Width – 3m Width

Patch 750x750	Norm. Load 3D Model	Corresp. Displ. (mm)	Norm. Load 2D Model	Effective Width (m)
1/4 Span - 1/4 Width - Stretcher - W 3m	0.2	0.097	0.10	1.92
	0.4	0.230	0.23	1.73
	0.6	0.405	0.34	1.74
	0.8	1.085	0.48	1.66
	1	4.429	0.58	1.71
1/4 Span - 1/4 Width - Stretcher - W 6m	0.2	0.106	0.09	2.29
	0.4	0.265	0.20	2.04
	0.6	0.491	0.30	2.01
	0.8	1.195	0.38	2.08
	1	3.984	0.44	2.25
1/4 Span - 1/4 Width - Stretcher - W 9m	0.2	0.126	0.09	2.11
	0.4	0.277	0.18	2.21
	0.6	0.603	0.30	2.03
	0.8	1.406	0.36	2.22
	1	3.783	0.39	2.59



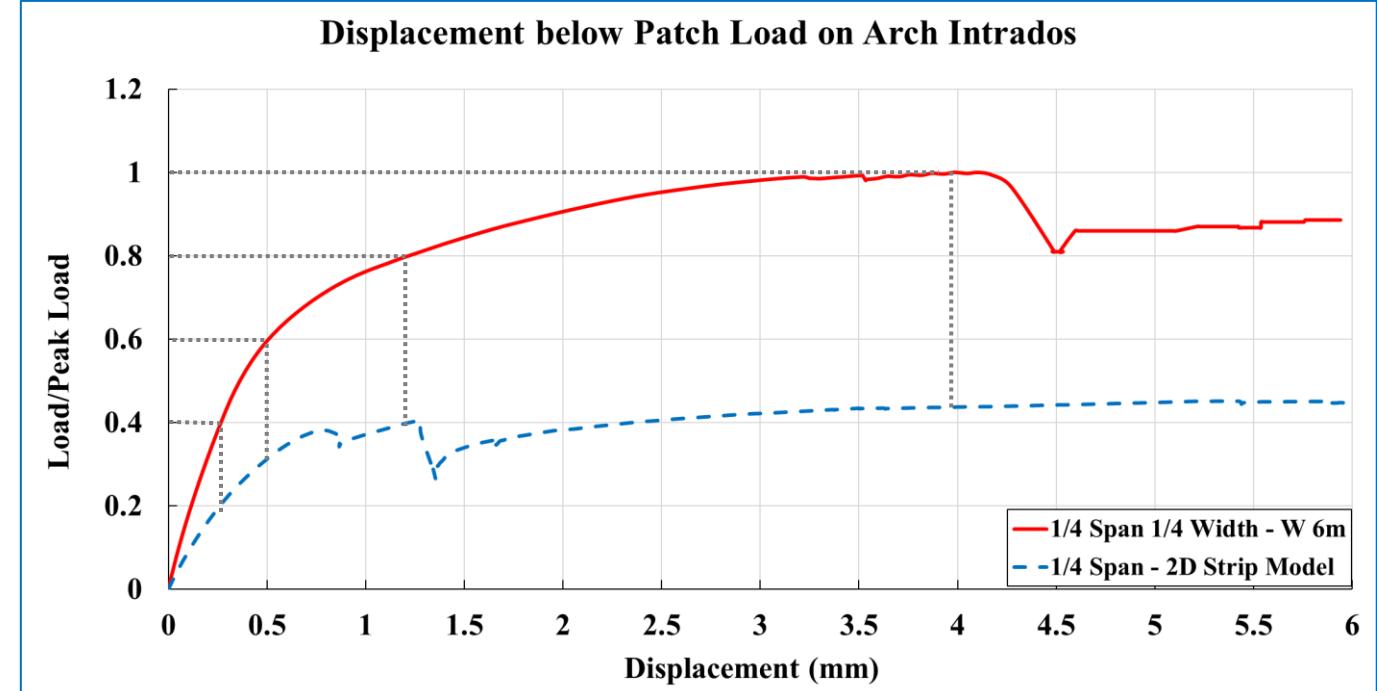
Numerical results

3D vs 2D models – effective width



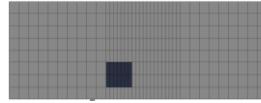
Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{4}$ Width – 6m Width

Patch 750x750	Norm. Load 3D Model	Corresp. Displ. (mm)	Norm. Load 2D Model	Effective Width (m)
1/4 Span - 1/4 Width - Stretcher - W 3m	0.2	0.097	0.10	1.92
	0.4	0.230	0.23	1.73
	0.6	0.405	0.34	1.74
	0.8	1.085	0.48	1.66
	1	4.429	0.58	1.71
1/4 Span - 1/4 Width - Stretcher - W 6m	0.2	0.106	0.09	2.29
	0.4	0.265	0.20	2.04
	0.6	0.491	0.30	2.01
	0.8	1.195	0.38	2.08
	1	3.984	0.44	2.25
1/4 Span - 1/4 Width - Stretcher - W 9m	0.2	0.126	0.09	2.11
	0.4	0.277	0.18	2.21
	0.6	0.603	0.30	2.03
	0.8	1.406	0.36	2.22
	1	3.783	0.39	2.59



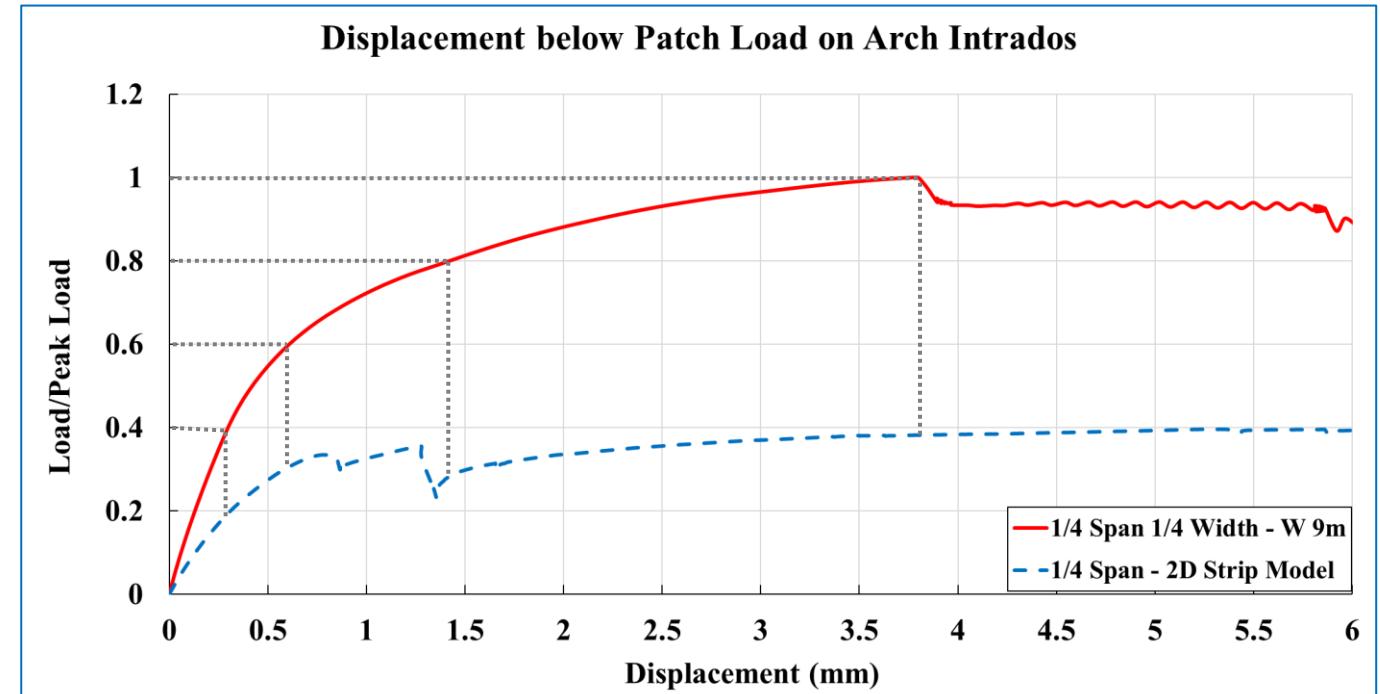
Numerical results

3D vs 2D models – effective width



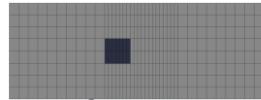
Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{4}$ Width – 9m Width

Patch 750x750	Norm. Load 3D Model	Corresp. Displ. (mm)	Norm. Load 2D Model	Effective Width (m)
1/4 Span - 1/4 Width - Stretcher - W 3m	0.2	0.097	0.10	1.92
	0.4	0.230	0.23	1.73
	0.6	0.405	0.34	1.74
	0.8	1.085	0.48	1.66
	1	4.429	0.58	1.71
1/4 Span - 1/4 Width - Stretcher - W 6m	0.2	0.106	0.09	2.29
	0.4	0.265	0.20	2.04
	0.6	0.491	0.30	2.01
	0.8	1.195	0.38	2.08
	1	3.984	0.44	2.25
1/4 Span - 1/4 Width - Stretcher - W 9m	0.2	0.126	0.09	2.11
	0.4	0.277	0.18	2.21
	0.6	0.603	0.30	2.03
	0.8	1.406	0.36	2.22
	1	3.783	0.39	2.59



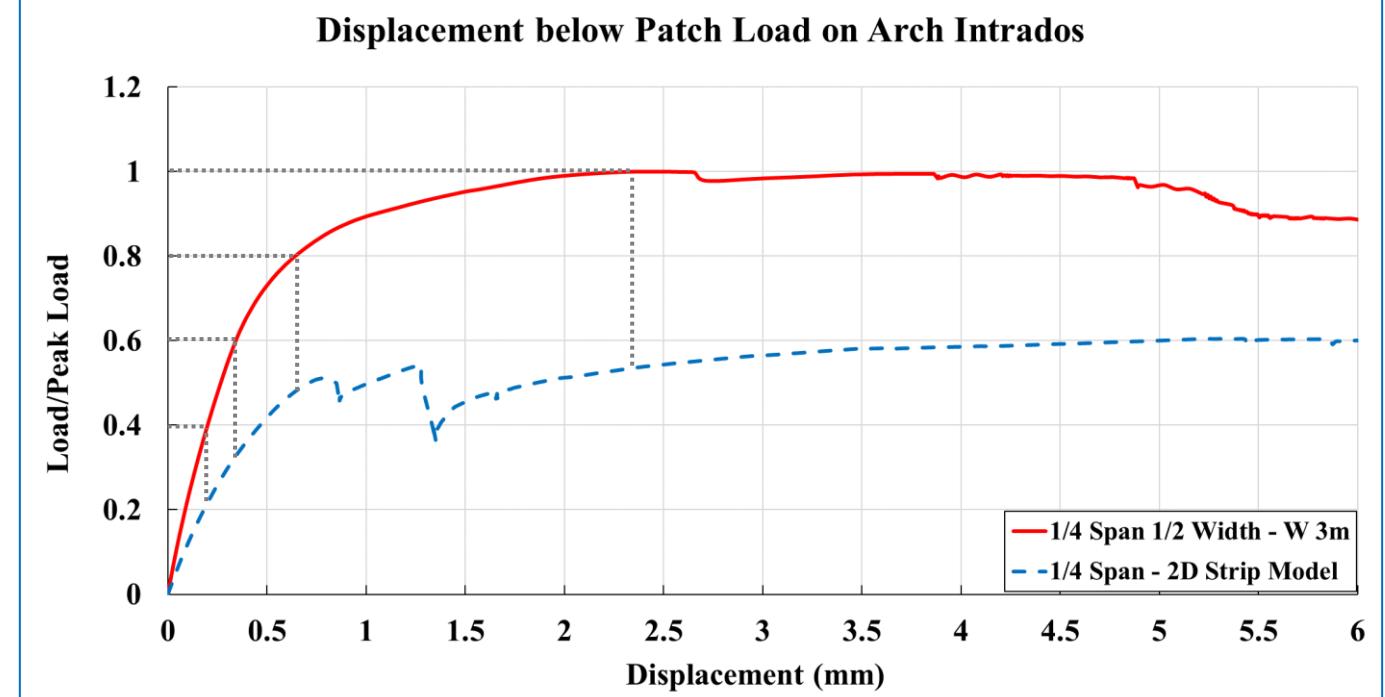
Numerical results

3D vs 2D models – effective width



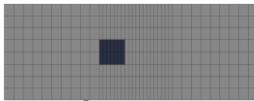
Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{2}$ Width – 3m Width

Patch 750x750	Norm. Load 3D Model	Corresp. Displ. (mm)	Norm. Load 2D Model	Effective Width (m)
1/4 Span - 1/2 Width - Stretcher - W 3m	0.2	0.087	0.10	2.03
	0.4	0.182	0.20	2.03
	0.6	0.341	0.32	1.90
	0.8	0.640	0.47	1.71
	1	2.407	0.53	1.88
1/4 Span - 1/2 Width - Stretcher - W 6m	0.2	0.124	0.08	2.40
	0.4	0.319	0.18	2.16
	0.6	0.663	0.29	2.07
	0.8	1.452	0.34	2.34
	1	3.924	0.36	2.74
1/4 Span - 1/2 Width - Stretcher - W 9m	0.2	0.125	0.09	2.26
	0.4	0.322	0.18	2.18
	0.6	0.671	0.29	2.06
	0.8	1.483	0.34	2.34
	1	3.169	0.35	2.89



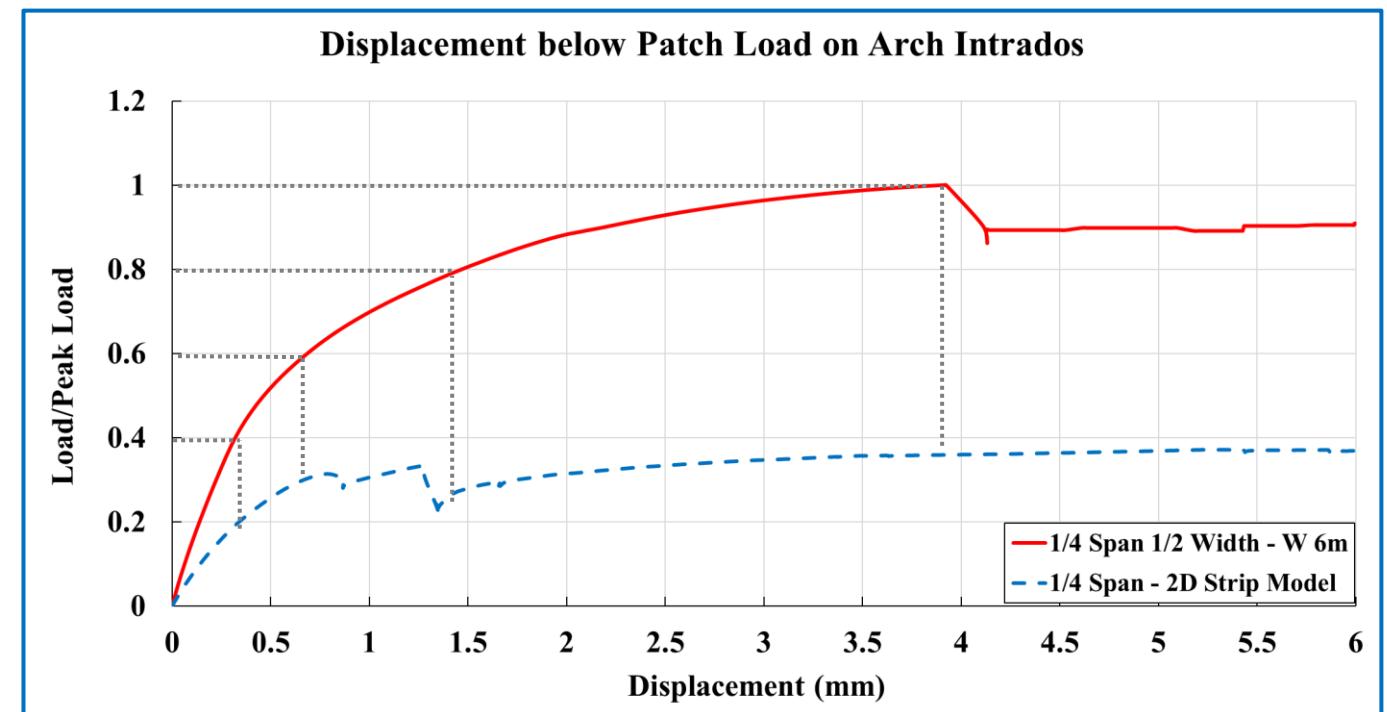
Numerical results

3D vs 2D models – effective width



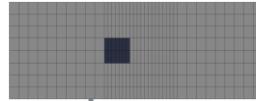
Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{2}$ Width – 6m Width

Patch 750x750	Norm. Load 3D Model	Corresp. Displ. (mm)	Norm. Load 2D Model	Effective Width (m)
1/4 Span - 1/2 Width - Stretcher - W 3m	0.2	0.087	0.10	2.03
	0.4	0.182	0.20	2.03
	0.6	0.341	0.32	1.90
	0.8	0.640	0.47	1.71
	1	2.407	0.53	1.88
1/4 Span - 1/2 Width - Stretcher - W 6m	0.2	0.124	0.08	2.40
	0.4	0.319	0.18	2.16
	0.6	0.663	0.29	2.07
	0.8	1.452	0.34	2.34
	1	3.924	0.36	2.74
1/4 Span - 1/2 Width - Stretcher - W 9m	0.2	0.125	0.09	2.26
	0.4	0.322	0.18	2.18
	0.6	0.671	0.29	2.06
	0.8	1.483	0.34	2.34
	1	3.169	0.35	2.89



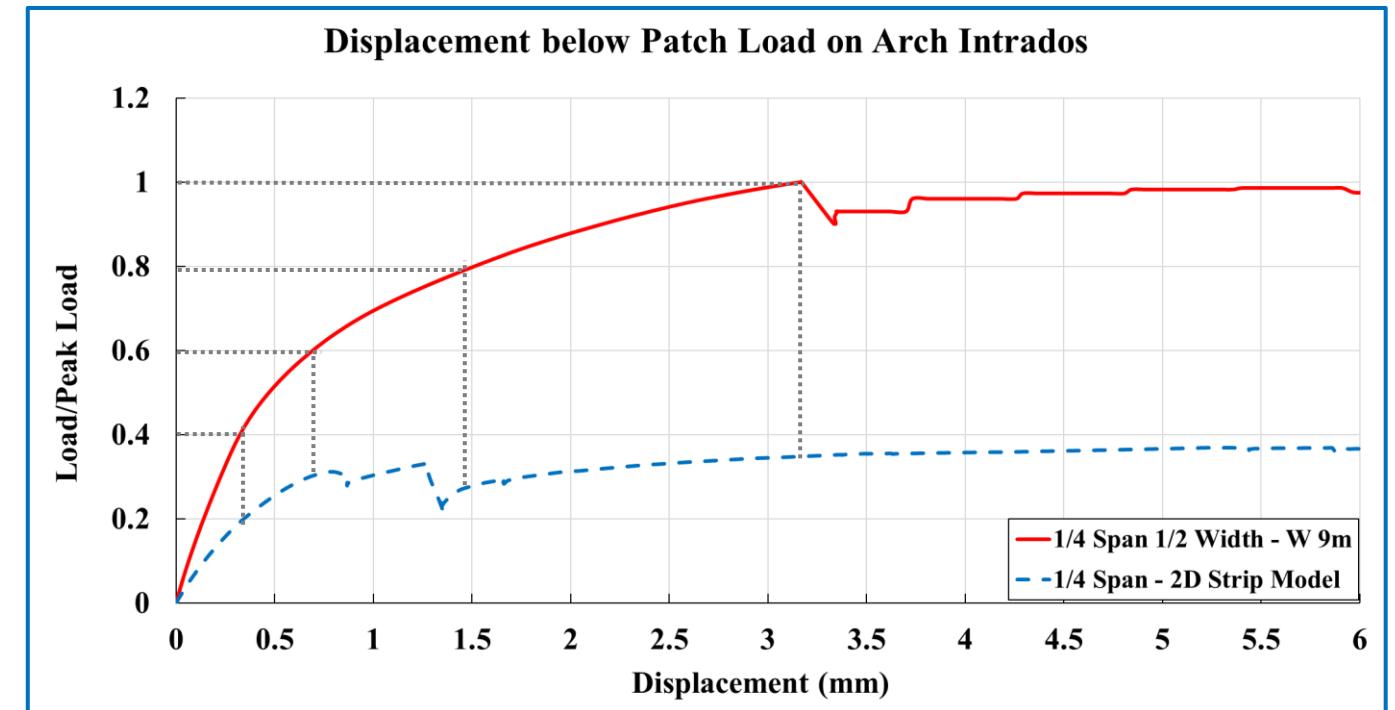
Numerical results

3D vs 2D models – effective width



Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{2}$ Width – 9m Width

Patch 750x750	Norm. Load 3D Model	Corresp. Displ. (mm)	Norm. Load 2D Model	Effective Width (m)
1/4 Span - 1/2 Width - Stretcher - W 3m	0.2	0.087	0.10	2.03
	0.4	0.182	0.20	2.03
	0.6	0.341	0.32	1.90
	0.8	0.640	0.47	1.71
	1	2.407	0.53	1.88
1/4 Span - 1/2 Width - Stretcher - W 6m	0.2	0.124	0.08	2.40
	0.4	0.319	0.18	2.16
	0.6	0.663	0.29	2.07
	0.8	1.452	0.34	2.34
	1	3.924	0.36	2.74
1/4 Span - 1/2 Width - Stretcher - W 9m	0.2	0.125	0.09	2.26
	0.4	0.322	0.18	2.18
	0.6	0.671	0.29	2.06
	0.8	1.483	0.34	2.34
	1	3.169	0.35	2.89



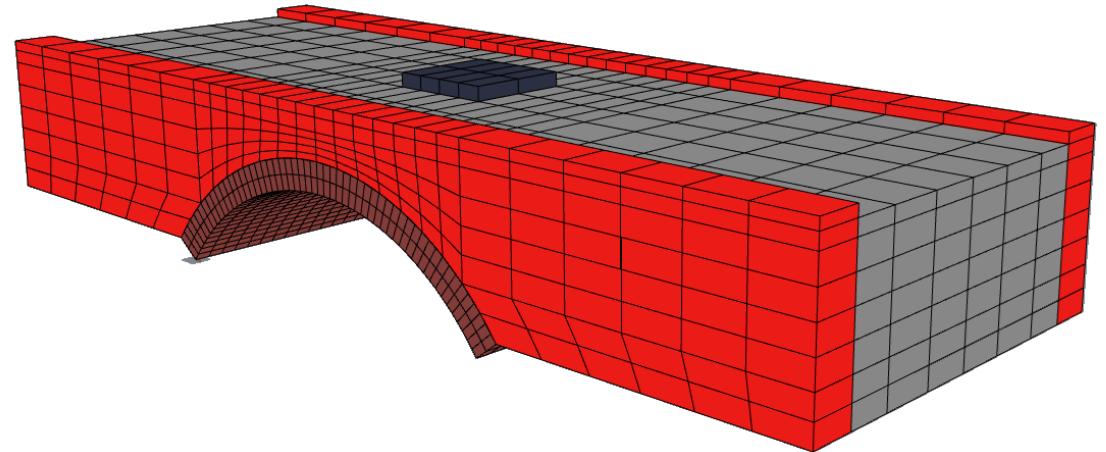
Model complexity study

Numerical results

3D Meso vs Macroscale Models



3D macroscale model



Material macroscale parameters for masonry*

Brick units		
E (Young's modulus)	N/mm ²	69000
v (Poison's ratio)		0.15
Mortar Interfaces		
E (Young's modulus)	N/mm ²	15900
n (Poison ratio)		0.15
Ψ_m (Dilatancy angle)	Deg	30
F _{t0} (Initial uniaxial tensile strength)	N/mm ²	0.21
F _c (Maximum uniaxial compressive strength)	N/mm ²	23.8
G _{ft} (Fracture energy in uniaxial tension)	Nmm/mm ²	0.01

Material parameters for backfill*

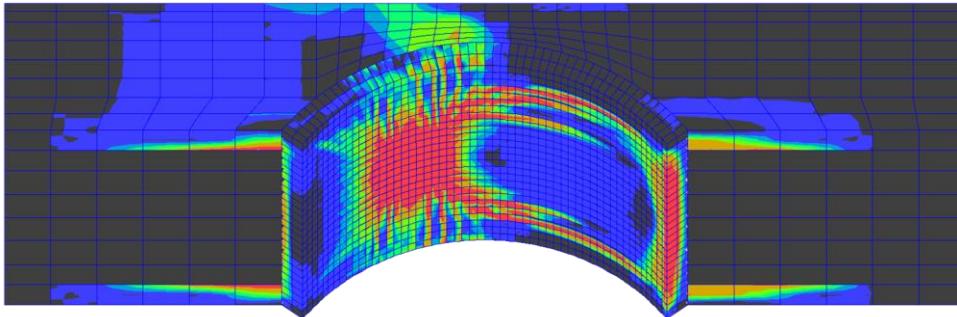
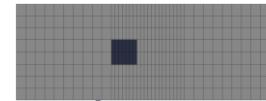
Backfill		
E _f (Young's modulus)	N/mm ²	200
v (Poison's ratio)		0.2
c (cohesion)	N/mm ²	0.001
ϕ_f (friction angle)	rad	1.047
Ψ_f (dilatancy angle)	rad	0.5236

*from material tests (Melbourne & Gilbert 1995) used for model validation

Numerical results

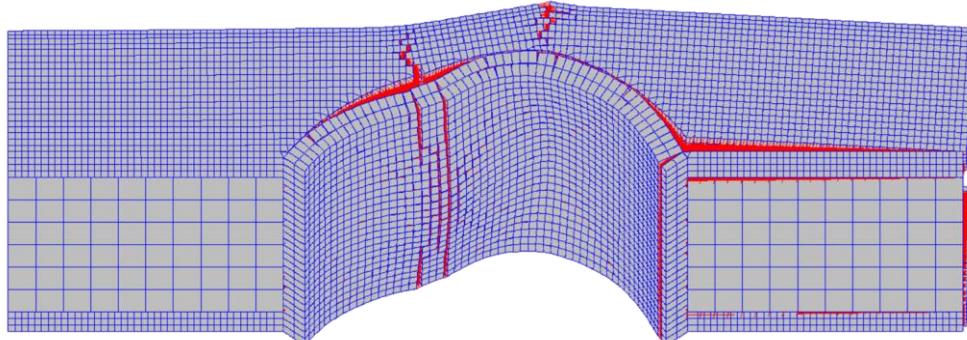
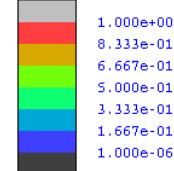
3D Meso vs Macroscale Models

Patch 750mm×750mm at ¼ Span ½ Width

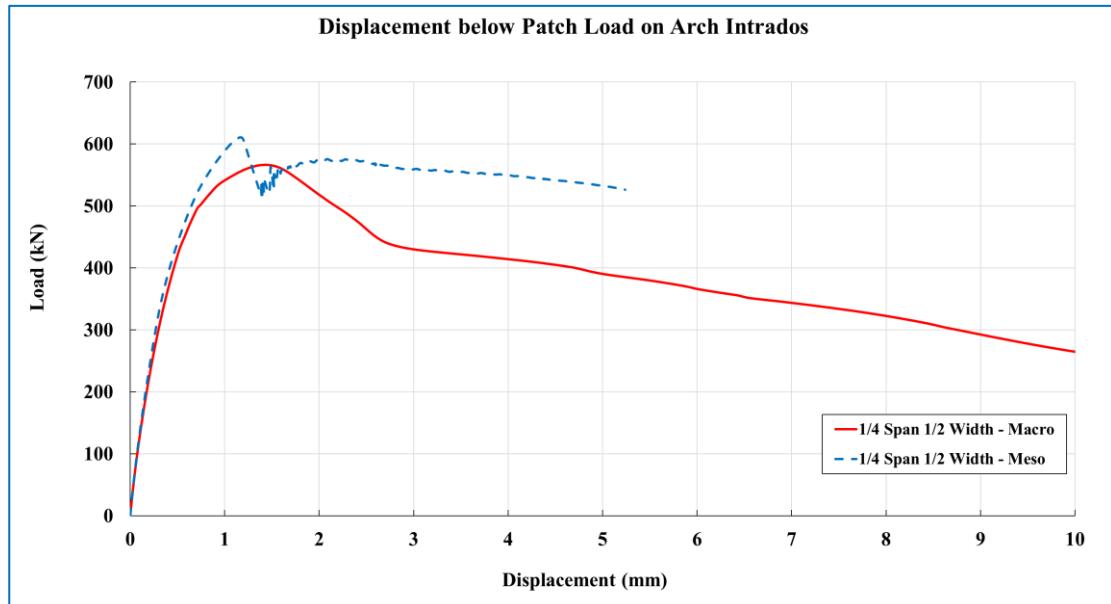


3D macroscale model

Damage



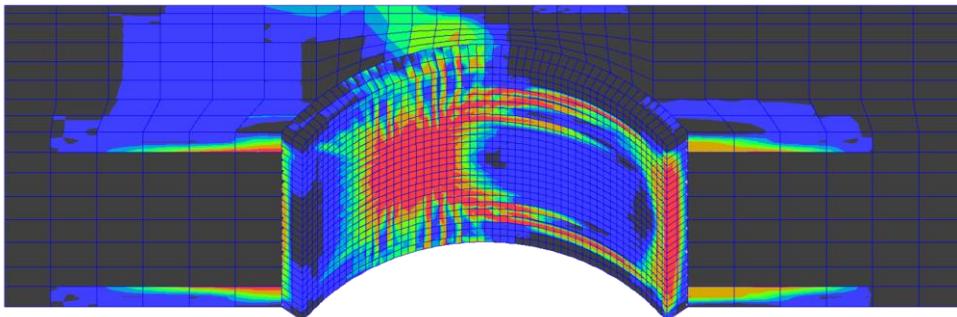
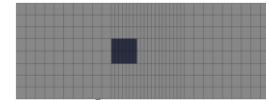
3D mesoscale model (stretcher)



Numerical results

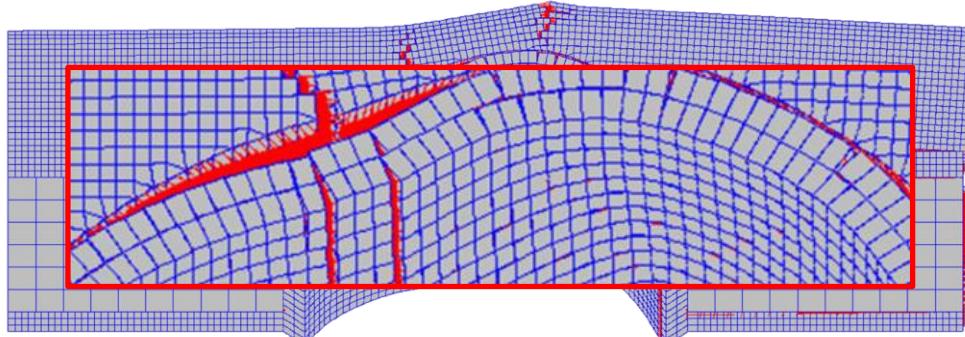
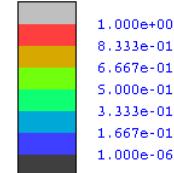
3D Meso vs Macroscale Models

Patch 750mm×750mm at ¼ Span ½ Width



3D macroscale model

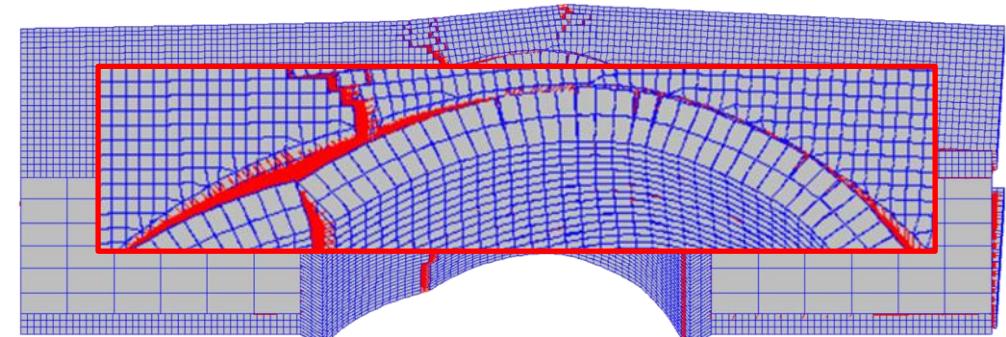
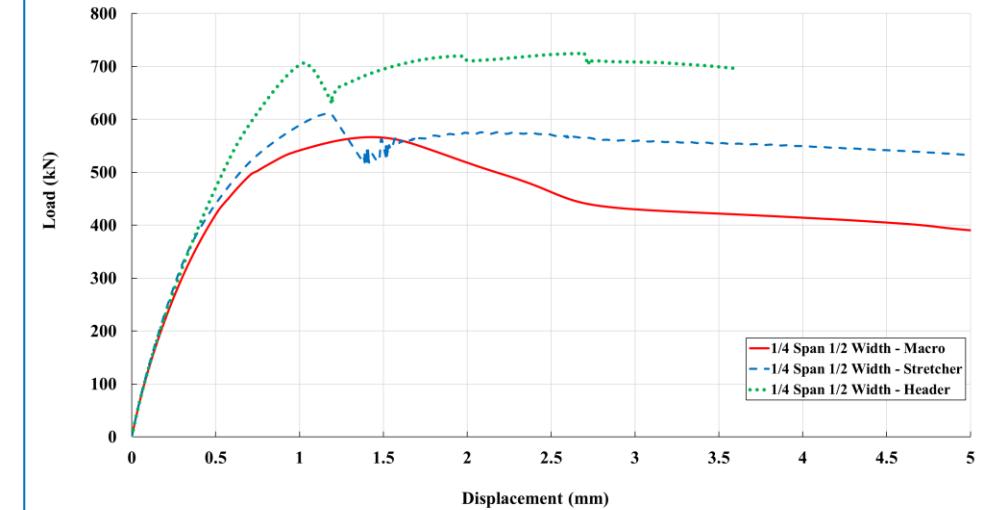
Damage



3D mesoscale model (stretcher)



Displacement below Patch Load on Arch Intrados

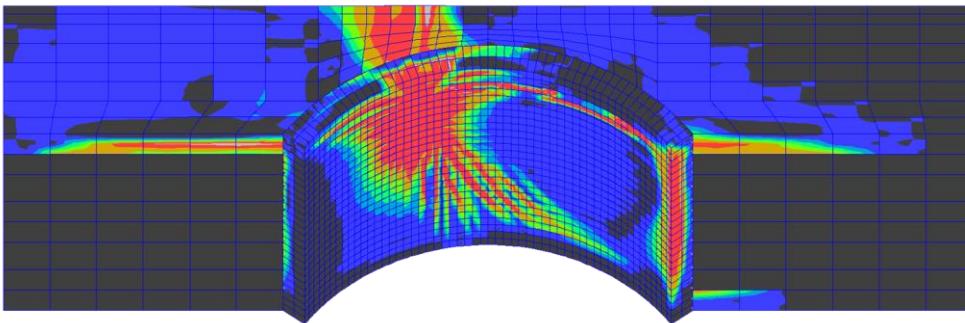
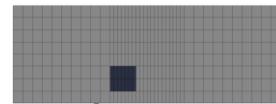


3D mesoscale model (header)

Numerical results

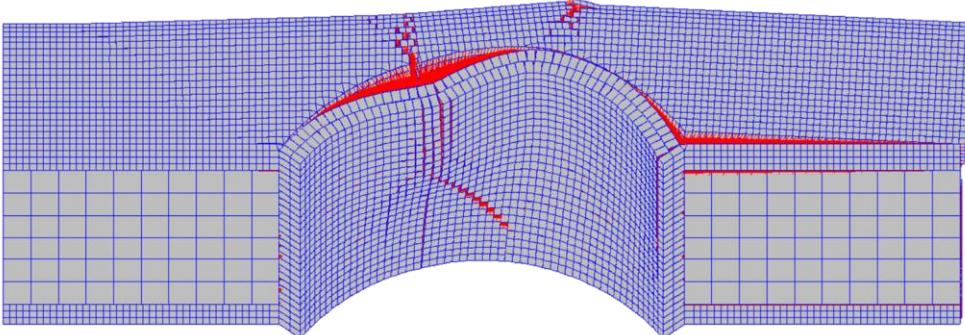
3D Meso vs Macroscale Models

Patch 750mm×750mm at ¼ Span ¼ Width

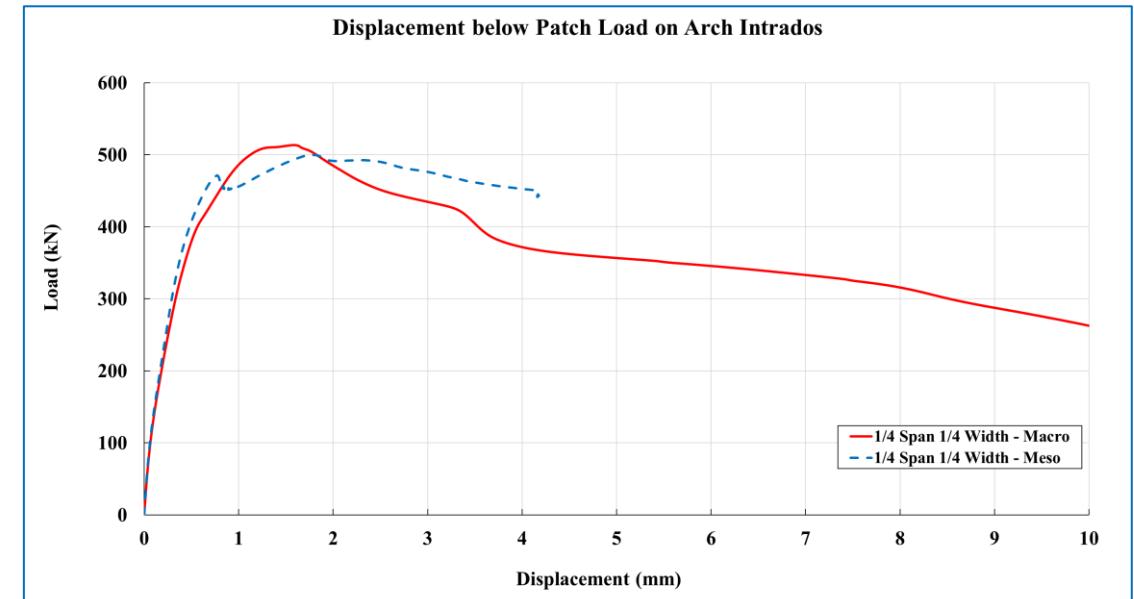


Damage

1.000e+00
8.333e-01
6.667e-01
5.000e-01
3.333e-01
1.667e-01
1.000e-06



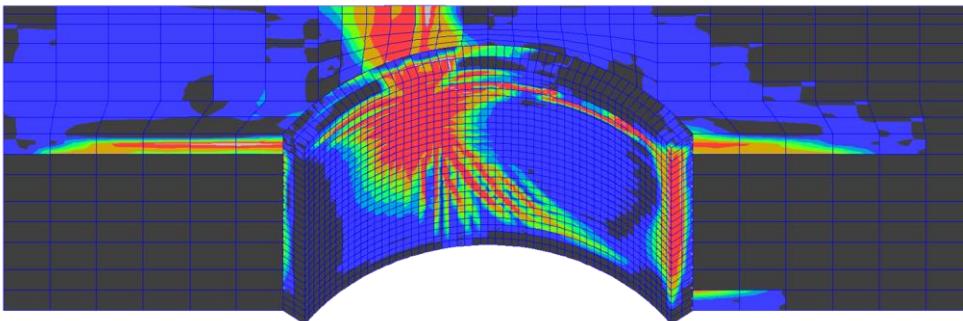
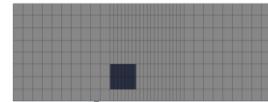
3D mesoscale model (stretcher)



Numerical results

3D Meso vs Macroscale Models

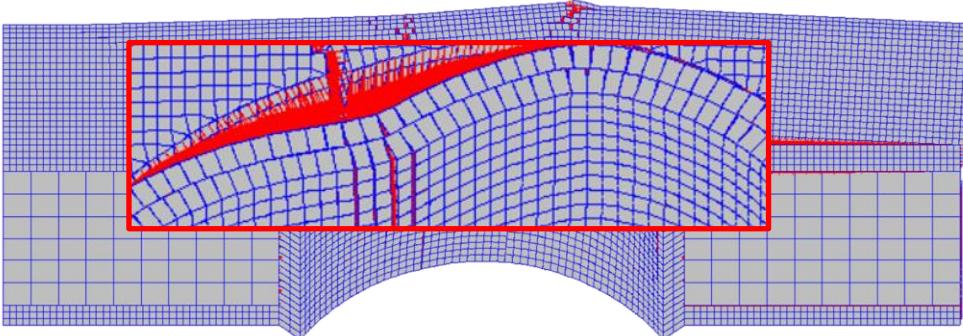
Patch 750mm×750mm at ¼ Span ¼ Width



Damage

1.000e+00
8.333e-01
6.667e-01
5.000e-01
3.333e-01
1.667e-01
1.000e-06

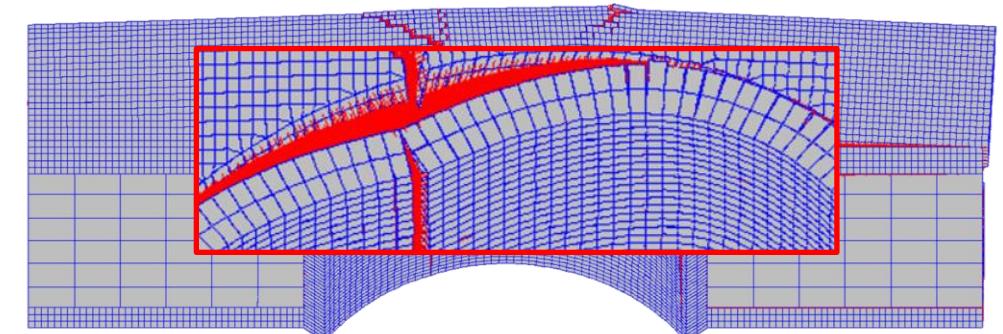
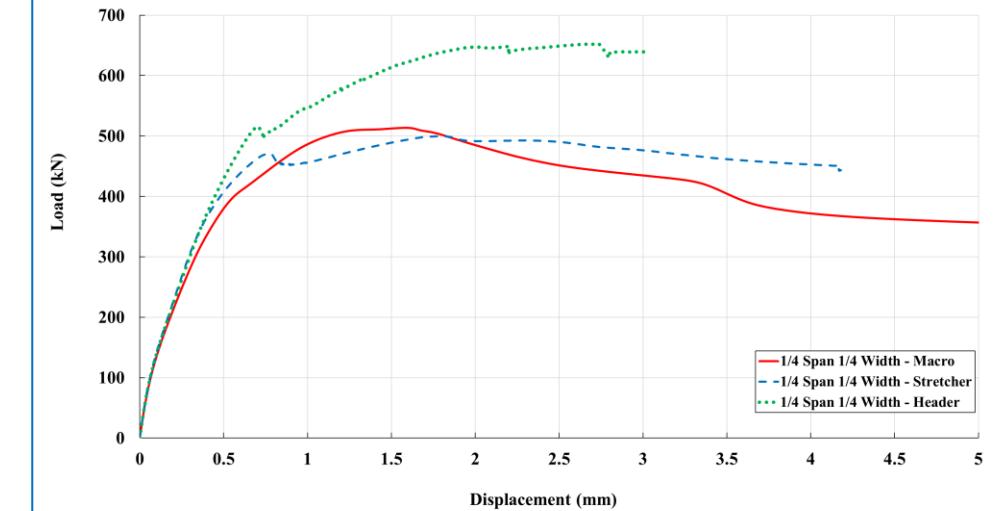
3D macroscale model



3D mesoscale model (stretcher)



Displacement below Patch Load on Arch Intrados

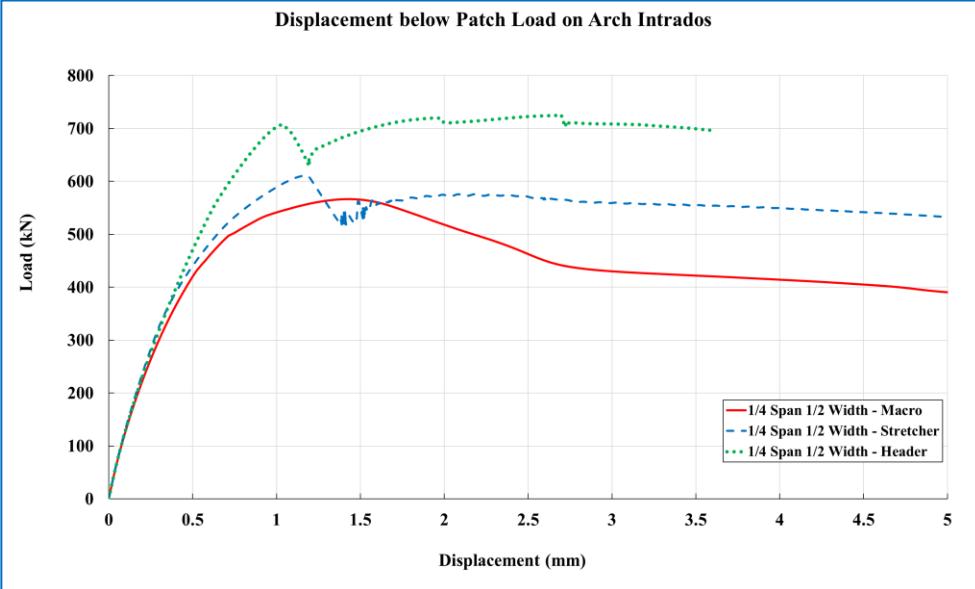
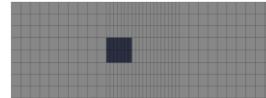


3D mesoscale model (header)

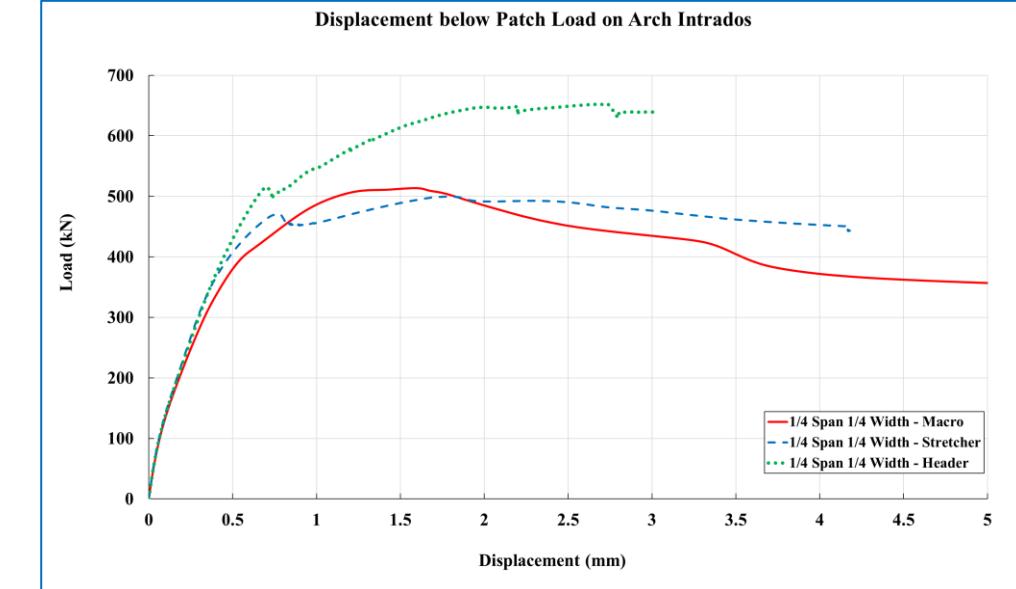
Numerical results

3D Meso vs Macroscale Models

Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{2}$ Width



Patch 750mm×750mm at $\frac{1}{4}$ Span $\frac{1}{4}$ Width



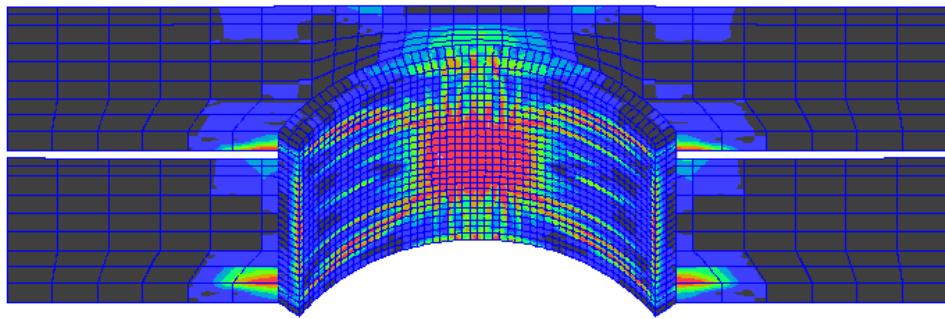
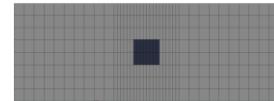
- Standard macroscale masonry models cannot represent failure modes associated with a specific masonry bond



Numerical results

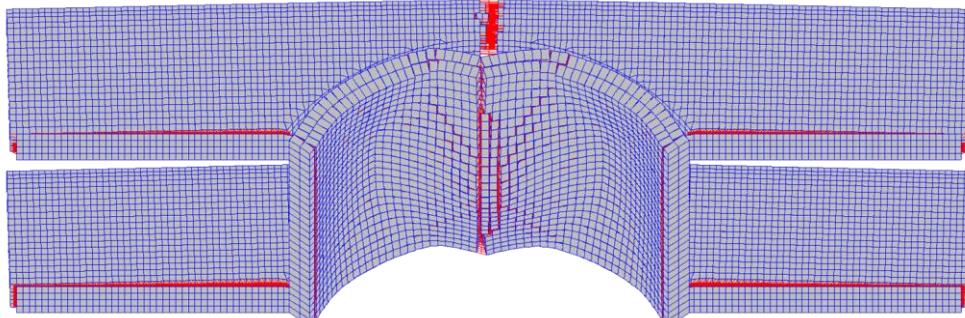
3D Meso vs Macroscale Models

Patch 750mm×750mm at ½ Span ½ Width

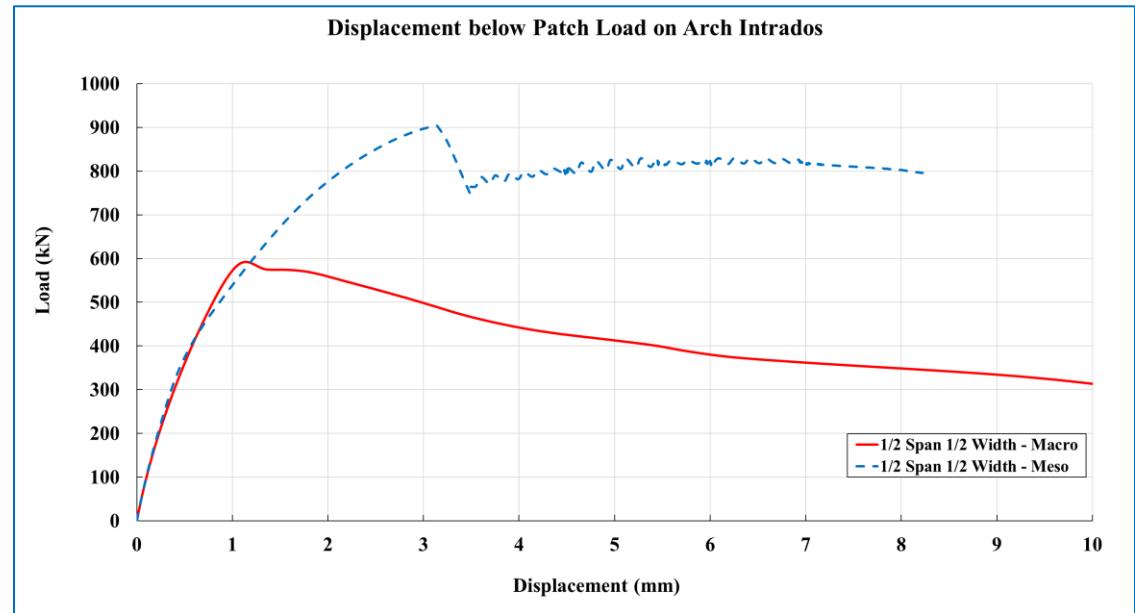


Damage

1.000e+00
8.333e-01
6.667e-01
5.000e-01
3.333e-01
1.667e-01
1.000e-06



3D mesoscale model (stretcher)

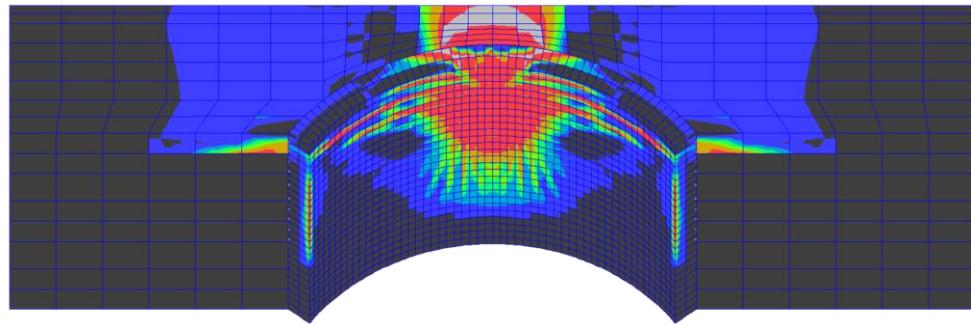
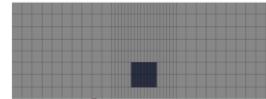


- Macroscale model predicts unrealistic punching shear failure in the arch under the load!

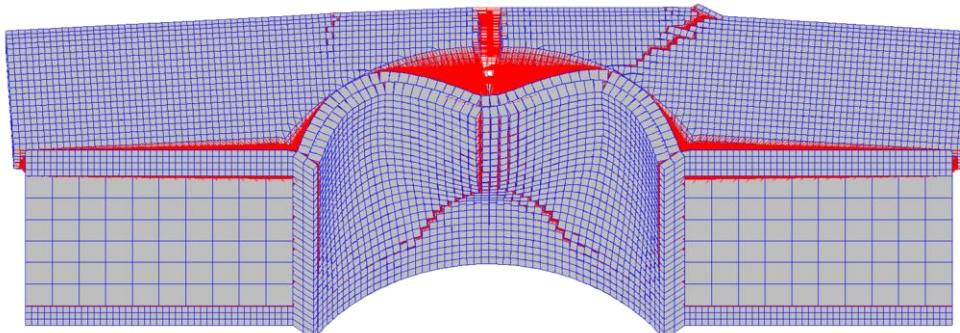
Numerical results

3D Meso vs Macroscale Models

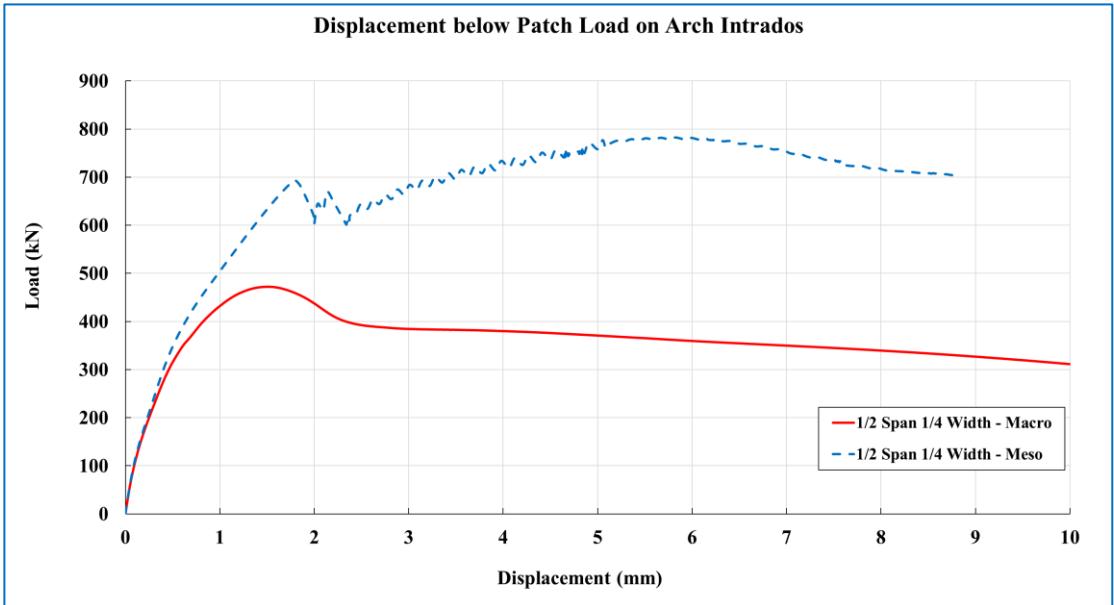
Patch 750mm×750mm at ½ Span ¼ Width



3D macroscale model



3D mesoscale model (stretcher)



- Macroscale model predicts unrealistic punching shear failure in the arch under the load!

Conclusions



Conclusions

- High-fidelity models provide baseline response data for the development of a consistent multi-level assessment framework and for the calibration of more efficient models.
- Safe assessment should consider the interaction between the different components, including spandrel walls. Weak spandrel walls may suffer early damage, hampering backfill confinement, and leading to reduced load capacity.
- The effective width used in 2D assessment models depends on load position and magnitude, backfill characteristics and on 3D arch behaviour.



Conclusions

- The use of nonlinear 3D models with simplified representations of the masonry can lead to inconsistent results. It can be mitigated by using enhanced anisotropic macroscale models (e.g. Pantò et al. 2022a), or refined calibration strategies (e.g. Pantò et al. 2022b).

References

S. Grosman, A.B. Bilbao, L. Macorini, B.A. Izzuddin, 2021, "Numerical modelling of three- dimensional masonry arch bridge structures", Proceedings of the Institution of Civil Engineers - Engineering and Computational Mechanics, Vol. 174, pages: 96-113.

B. Pantò, S. Grosman, L. Macorini, B.A. Izzuddin, 2022a, "A macro-modelling continuum approach with embedded discontinuities for the assessment of masonry arch bridges under earthquake loading", Engineering Structures, Vol. 269, 114722.

B. Pantò, C. Chisari, S. L. Macorini, B.A. Izzuddin, 2022b, "A hybrid macro- modelling strategy with multi-objective calibration for accurate simulation of multi-ring masonry arches and bridges", Computers & Structures, Vol. 265, 106769.